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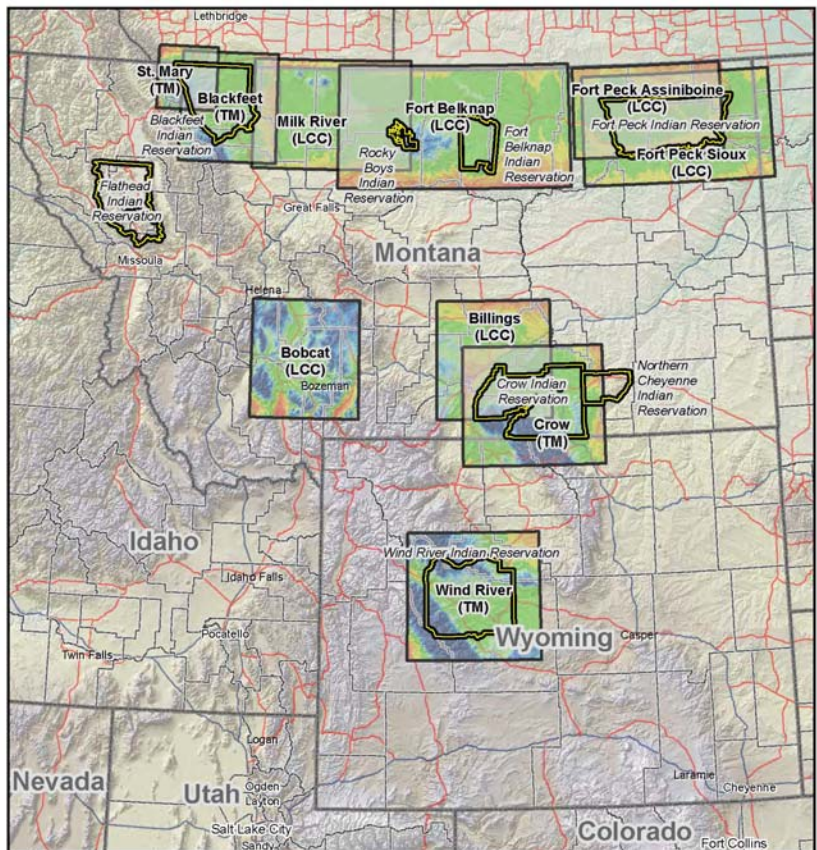
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# Rocky Mountain Tribal Coordinate Reference System Handbook and User Guide

For the  
**Blackfeet  
Crow  
Fort Belknap  
Fort Peck  
& Wind River Reservations**



Version - v1.0  
30 September 2014

## **Abstract**

This document contains the history, development, best practice methods, and technical creation of a new coordinate system for the Rocky Mountain Tribal areas. The Rocky Mountain Tribal Coordinate Reference System (RMTCRS) is based on a series of 'low distortion' map projections (zones) whose parameters have been defined such that lineal distortion is very minimal for certain geographic areas. Each zone has been optimized by design, to be useful for surveying, engineering, GIS, and cartographic mapping, where distances computed between points on the grid coordinate system will closely represent the distances physically measured between the same points on the ground within published zone tolerances. It is important to realize that rectangular grid coordinates for all of the RMTCRS map projections may now be calculated with formulas through computer programs that would have seemed too complicated in the past, but now may be considered to be a routine exercise. These same computer programs also make it a relatively simple procedure to complete transformations, moving the coordinates of a point or group of points from one coordinate system referenced to one datum, into coordinates referenced to a different datum for a given epoch. While having numerous state coordinate systems may seem cumbersome at first, actual user application through highly precise GNSS and terrestrial measurement devices provide for a level of mapping accuracy that is beneficial to all mapping professionals.

## **Revision History**

This document has been developed by Northern Engineering & Consulting, Inc. (NECI) from the Oregon Coordinate Reference System (OCRS) Handbook and User Guide, Version 2.00, Mark L. Armstrong, 25 March, 2011. NECI revised the OCRS Handbook in accordance with Montana and Wyoming survey systems and the coordinate reference systems developed for the study areas, but much of the original information contained in the OCRS remains in this publication.

Draft v1.0 (First draft for inside review)

19 May, 2014  
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Draft v1.1 (Updated draft for working group, vendor, and potential users review)

18 June, 2014  
-----

First Release v1.0 (Printed version handout for "Rollouts" and for MtWyTLC Presentation)

30 September, 2014  
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*This document is formatted for double sided printing.*

## Acknowledgements

I would like to acknowledge and thank the people who contributed to the development of the Rocky Mountain Tribal Coordinate Reference System.

**Michael Dennis**, our Low Distortion Projection expert, whose tremendous knowledge, expertise, and the amazing software tools he developed were instrumental to this undertaking. Mr. Dennis as the “Greatest Generation” would say is battle tested and when attempting change on the magnitude we have hoped for having Mr. Dennis at our side has been a key to accomplishing as much as we have.

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**Mark Armstrong**, Oregon’s Geodetic Advisor, who led the development of the Oregon manual, has advised the Rocky Mountain team as well. Mark has provided valuable insight and knowledge not only on Low Distortion Projection but he has conducted Low Distortion Projection trainings at our state surveyor’s association conferences. He has helped pave the way for Low Distortion Projection acceptance among the Montana and Wyoming professional land survey community.

**Jay Springer**, Northern Engineering & Consulting, worked closely with Mr. Dennis in development and design of the Milk River and Crow Low Distortion Projections. His love of surveying has made the low distortion venture enjoyable. Further, Mr. Springer has advocated Low Distortion Projection implementation through his use of the projections in survey and design.

**Rich Jensen**, Sanderson Stewart, developed the Billings and assisted with the Bobcat Low Distortion Projections. Mr. Jensen’s support, resources and promotion of low distortion projections have been instrumental to acceptance and implementation of the projections.

The **Tribal Team** is who has been instrumental in progression of our grass roots movement includes:

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Finally, the **Oregon Technical Development Team**, who developed, documented, and implemented the OCRS zones. The group blazed the trail and gave us permission to use their User Guide and Handbook as a template. Without their leadership, guidance, and template we would not have known where to begin. The Oregon Technical Development Team includes:

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## **Living Document**

This RMTCRS Handbook and User Guide is designed to be a 'living document' and will be updated with information and additional RMTCRS coordinate systems as new low distortion map projections are developed over time.

The RMTCRS was created with public money and effort for the benefit of surveying, engineering, GIS, and mapping professionals on the Blackfeet, Crow, Fort Belknap, Fort Peck, and Wind River Indian Reservations in Montana and Wyoming. The Rocky Mountain Tribal areas are among several states that have created new coordinate systems based on 'low distortion' map projections.

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# Chapter 1 History and Development of the RMTCRS

## 1.1 History and Development of the Rocky Mountain Tribal Coordinate Reference System (RMTCRS)

The utilization of electronic survey data by surveyors and GIS professionals is bringing awareness of the need for higher accuracy when working with measurements on the earth and their representation in electronic databases and on paper. Modern GIS and surveying software now brings the opportunity to create low distortion map projections and coordinate systems that can relate closely to distances measured on the ground. The function of low distortion projections is to minimize the distortions of distances, areas and to a lesser extent azimuths and angles. These distortions are ever present because we live on a semi-round spheroid, and are presented with the impossibility of representing a curved surface on a plane without distortion. We can minimize that distortion by creating a mathematical model (map projection) that will allow us to work in a coordinate grid where calculated positions and distances are represented closely by the same positions and distances we measure on the ground. For mapping and GIS professionals, low distortion projections may dramatically reduce the need to ‘rubber-sheet’ data sets to make features fit. Now both survey and GIS data can co-exist without either dataset being degraded.

### 1.1.1 The Beginning

For many years surveyors in Montana and Wyoming have been looking for a better way to deal with map distortion other than the currently used State Plane Coordinate Systems. In 2009, John Smith, Shoshone and Arapahoe Department of Transportation, Tribal Roads Director, gave direction to investigate the use of ‘low distortion’ projections to determine the pros and cons of their use. At the 2010 Montana Association of Registered Land Surveyors conference Mr. Gladstone and Mr. Robertson, on behalf of Fort Peck, met with Curt Smith, NGS advisor, to discuss the subject and we were directed to Mr. Michael Dennis. We soon learned the surveying process could be standardized and simplified and that if we standardized the system a surveyor no longer needed to be a student of geodesy to use a GPS survey instrument to measure a line on the ground. We had an opportunity to create a standard coordinate system that could be used by all tribal surveyors and if we published the system it could be shared and beneficial to all members of the survey and engineering community. Mr. Smith gave direction to proceed. In 2011 Fort Peck formally joined the mission and the project became a tribal mapping project. The Blackfeet and Fort Belknap reservations joined in 2012 and the Crow in 2014. As our team learned more about the national survey system we learned about our regional survey foundation short comings therefore additional phases were added to our tribal mapping project.

The six phase Tribal Mapping project is described below:

- **Phase 1-Low Distortion Projection (LDP) Creation**-As described in this document, LDP’s were established on each reservation to minimize map projection errors arising from the use of State Plane Coordinate Systems.
- **Phase 2-Control Point Establishment in the Tribal Coordinate Systems**-Ground based control points have been established for project control and quality assurance. NGS generally refers to this type of control as passive control and is no longer supporting it. The NGS movement has been toward CORS therefore we’ve added phase 4 to the mapping project.
- **Phase 3-Tribal Mapping Handbook Creation**-This handbook was created to guide users on LDP use, GPS input, and use in GIS systems.

- **Phase 4-Continuously Operating Reference Station (CORS) Establishment**-Static, survey grade GPS receivers will be established to provide access to the National Spatial Reference System (NSRS) to precisely identify latitude, longitude, and elevation.
- **Phase 5-Real Time Network (RTN) Establishment**-RTN stations will link to CORS to provide real-time data corrections and allow accurate GPS data to be collected in the field.
- **Phase 6-Survey Grade Data Collection and Compilation**-The mapping system will be used to collect highly accurate, survey grade data and compile the data in GIS systems for shared use.

## 1.2 The RMTCRS Technical Development Team

The Tribal Team was formed by tribal transportation directors, engineers and GIS users exploring interest in the tribal mapping project in meetings and initial rollouts through 2010. For the names of the tribal project managers and other contributors, see the acknowledgements inside the front cover. See Figure 1.2 for a graphic representation of the time line beginning in 2009 and continuing to the state and tribal adoption of the projections. The participating tribes and NECI worked closely with Michael Dennis of Geodetic Analysis, LLC to construct projections through a refined iterative process leading to a final optimized solution for each geographic area.

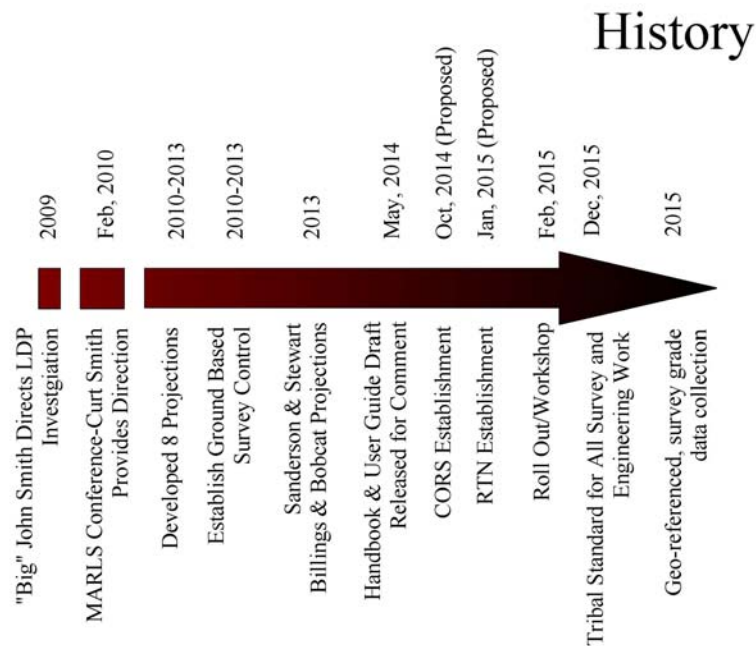


Figure 1.2: Historical Timeline for the RMTCRS

## 1.3 RMTCRS 'Best Practice' Goals

Best practices used for the RMTCRS program were developed by the Oregon CRS Technical Development Team in 2009 and adopted with minor edits by the RMTCRS team in 2010. The 'Best practices' focus on the critical elements lead to the creation of new map projection zones. These 'best practices' continued to evolve during the process and are currently listed below.

1. The goal was established to use 1:50 000 ratio =  $\pm 20$  ppm for each reservation [as big as zones as possible and still meet these criteria. No criteria difference between urban (local) and rural (regional) areas].
2. Use common and easy to implement map projections: Lambert, Transverse Mercator, with the Oblique Mercator (Rectified Skew Orthomorphic) added for special cases.
  - a. Vendor software needs to support these projections. The team is coordinating with vendors letting them know that new coordinate systems are under development.
3. The RMTCRS system would not require a site calibration (localization) by a surveyor for horizontal positioning in each projection zone coordinate system.

4. Each zone would have a positive NE coordinate system.
5. The false Northing's and Easting's for each zone would be designed to not conflict with one another and be markedly different than State Plane coordinates.
6. Units: (meters) - Metric units for map projection parameters and individual users may project into desired units. Montana users project to international feet and Wyoming users project to US Feet.
7. The RMTCRS zones will be referenced to the National Spatial Reference System (NSRS). This is currently defined geometrically as NAD 83 (GRS-80 ellipsoid) and it will follow the NGS path (new datum definitions') in future. The projection parameters will not be affected by a specific realization of NAD 83, since all of these realizations reference the GRS 80 ellipsoid.
8. Projections created should be referenced to NAD 83 'generically' with specific realization of NAD 83 (such as HARN, CORS96 or NSRS2007) stated in the metadata associated with the observed project datasets.
9. The method used to create each zone will not involve scaling the ellipsoid. Scaling modifies GRS-80, making the resulting projection not compatible with NAD 83.
10. If an existing low distortion projection already exists it will be reviewed by the Technical Development Team to see if it meets these 'best practices' and also provides for the greatest available  $\pm 20$ ppm coverage for the area under consideration.
11. The vertical datum will be the current NAVD 88, but will also follow the NGS lead adopting the future NAVD based on a pure gravimetric geoid (via the GRAV-D Project). The geoid model used is part of the metadata belonging to a full coordinate system; however the geoid is independent of the RMTCRS projection zone parameters.
12. The development of the RMTCRS system will include parameters for each zone that will be included in a future published Handbook and User Guide.
13. No artificial political boundaries will define the limits of a particular zone. Each zone will be defined by latitude and longitude limits, but may include the option to modify the zone limits to match key areas or include political boundaries (will try not to break populated areas into two zones).
14. Interact with NGS in the future to develop:
  - a. Standard methodology for low distortion project zone development.
  - b. In the future suggest the NGS develop an automated software tool for creating low distortion projection coordinate systems.
  - c. Document/register/catalog zones on the NGS website.
  - d. Discuss the possibility of RMTCRS and other state legislated zones being included on NGS datasheet output files, including OPUS output results.
15. Involve stakeholders in the review of the RMTCRS development by giving presentations etc. (local users: MARLS, PLSW, MWTL, ASCEMontana/Wyoming, GIS groups, MSU, tribal colleges, etc.)
16. Involve software vendors so they can include the RMTCRS zones when they update their software.
17. The size of each zone to be determined when created. Zones will cover as large an area as possible and still meet the distortion criteria, so as to minimize the total number of zones.
18. For Lambert Conformal Conic (LCC) zones, the Latitude of grid origin shall be the same as the standard parallel chosen.
19. Each zone must have unique coordinate system origins that differ from one another by a significant amount so as not to be confused with one another.

### 1.4 Why State Plane Coordinate Systems are Deficient for Certain Modern Day Uses

The State Plane Coordinate System was first studied in 1933 by the U.S. Dept. of Commerce, Coast and Geodetic Survey to simplify geodetic calculations and avoid complex ellipsoid calculations. The Montana State Plane Coordinate System is a single zone system based on the Lambert Conformal Conic Projection. The Wyoming State Plane Coordinate System is based on the Transverse Mercator Projection and consists of four zones to minimize distortion. The maximum distortion (with respect to the ellipsoid) was kept to approximately one part in 9,500 (105 parts per million)<sup>(5)</sup>. This distortion error occurs when these zones are constructed for mapping purposes and it is because of this, that the state plane system presents the following issues for the surveying and GIS community:

- Does not represent ground distances except near sea level elevations (along the coast and major river systems) and near the standard parallels.
- Does not minimize distortion over large areas and varying elevations.
- Does not reduce convergence angles.
- Does not support modern datum and geoid grid reference frames.

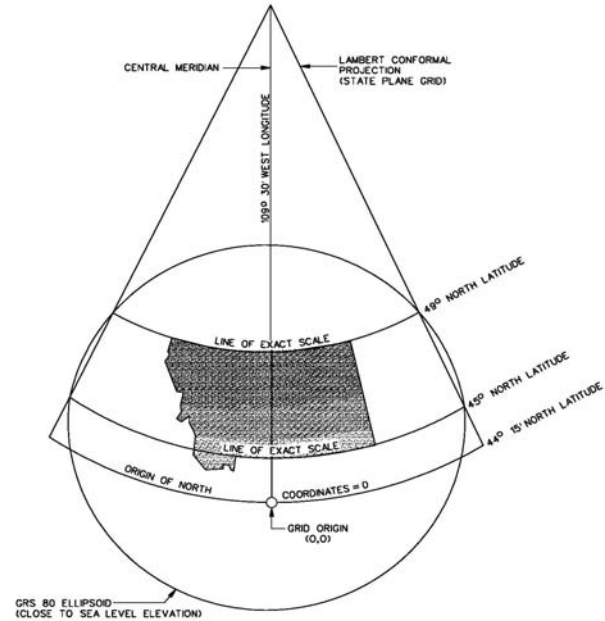


Figure 1.4.1: Montana State Plane Two parallel Lambert Conformal Conic Projection layout

Currently State Plane coordinates are available for all of Montana and Wyoming's horizontal control points that reside in the National Geodetic Survey (NGS) Integrated Database (datasheets) and are also generated for all points submitted to the NGS Online Positioning User Service (OPUS). The State Plane Coordinate Systems still maintains some limited

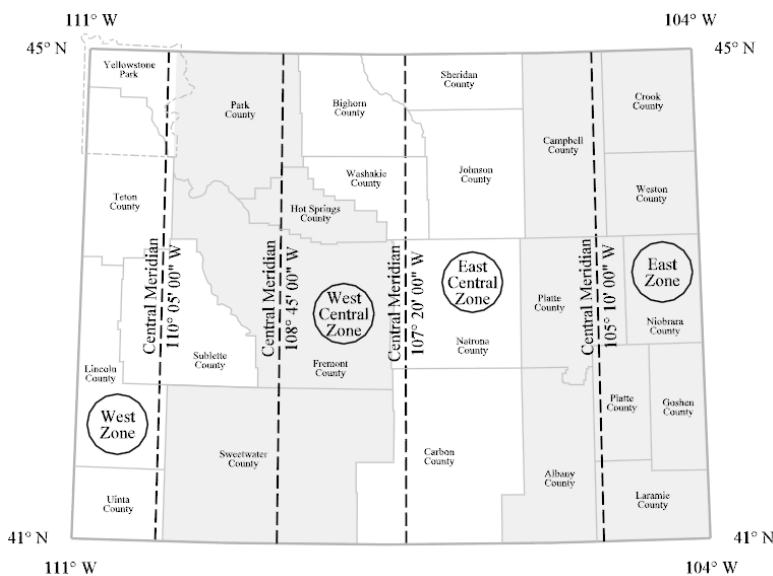
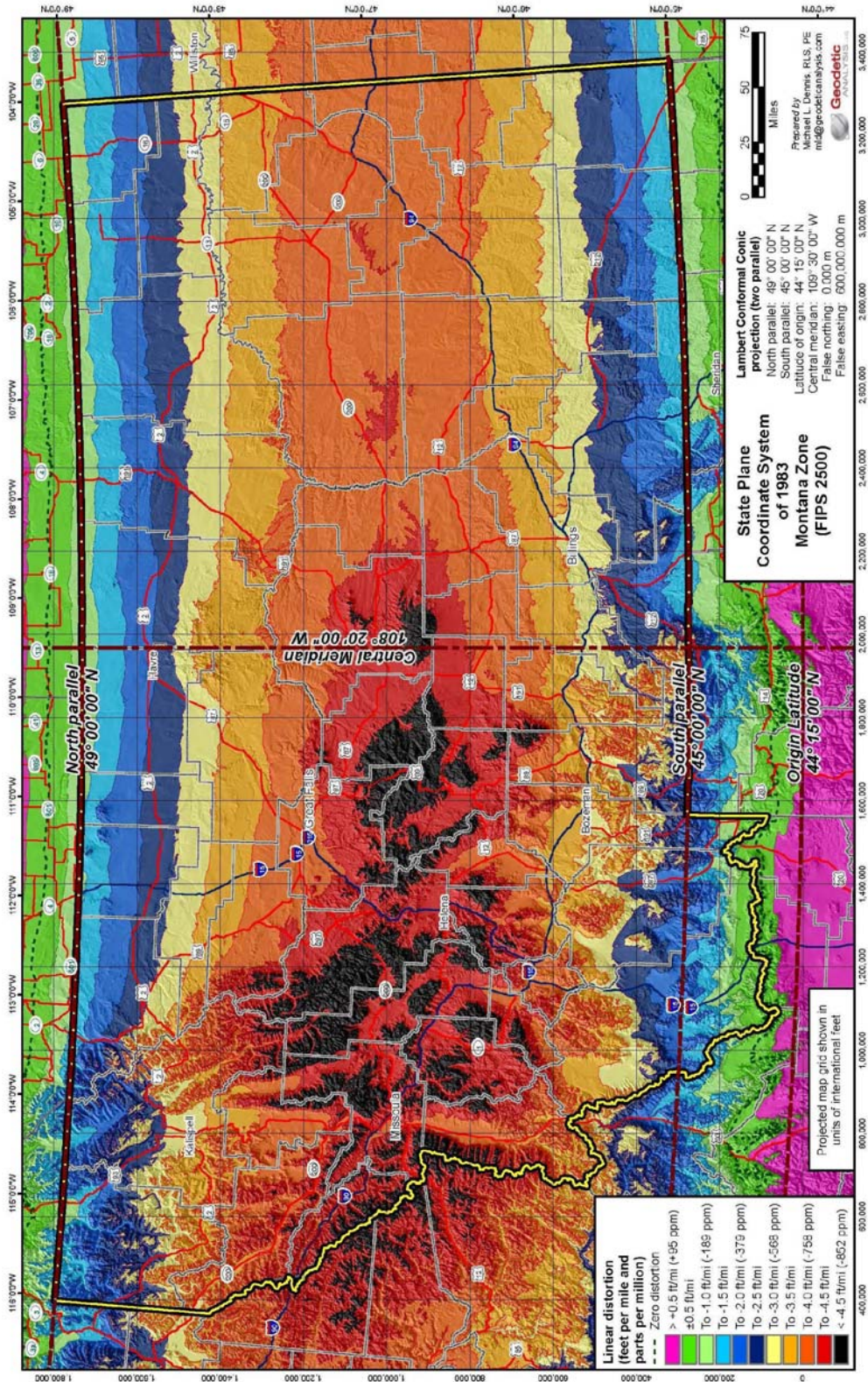


Figure 1.4.2: Wyoming Transverse Mercator State Plane Coordinate System Zones

advantages for general surveying and mapping (GIS) at a statewide level, such as depicting physical, cultural, and human geography over large areas of the state. It also works well for mapping long linear facility lines such as highways, electrical transmission, and pipelines, which crisscross the state. The State Plane Coordinate System provides for a common reference (map projection) for conversions (transformations) between other coordinate systems including the zones of the RMTCRS. The Figure below (Figure 1.6.0.1) depicts the total linear distortion (at the topographic surface of the Earth) for Montana. Note high distortion (greater than -4.5 feet/mile) occurs in the west central part of the state in areas of high elevation, where areas of low distortion occur near the north and south state lines at the parallels.

Figure 1.4.3



### 1.4.1 State Plane Coordinate System Definitions

Montana and Wyoming State Plane Coordinate Systems are defined as follows in Table 1.1, below.

Table 1.1

State Plane Zone	Zone Number	Projection Type	Central Meridian	Latitude of Origin	Standard Parallel (South)	Standard Parallel (North)	False Easting (m)	False Northing (m)	Max Scale Error*
Wyoming East	4901	Transverse Mercator	-105° 10' (W)	40° 30'	-	-	200,000	0	
Wyoming East Central	4902	Transverse Mercator	-107° 20' (W)	40° 30'	-	-	400,000	100,000	
Wyoming West Central	4903	Transverse Mercator	-108° 45' (W)	40° 30'	-	-	600,000	0	
Wyoming West	4904	Transverse Mercator	-110° 05' (W)	40° 30'	-	-	800,000	100,000	
Montana	2500	Lambert Conformal Conic 2 Standard Parallel	-109° 30' (W)	44° 15'	45° 15'	49°	600,000	0	<-852 ppm

\*Note: This maximum scale error is distortion with respect to the ellipsoid, not the topographic surface, and occurs along the central parallel. The actual distortion at the topographic surface is typically greater, and it changes at a rate of 4.8 ppm per 100-ft change in height.

Max scale errors have not yet been determined for Wyoming State Plane Zones.

## 1.5 Local Datum Plane Coordinate (LDPC) Method vs. Low Distortion Projection Method

### 1.5.1 Local Datum Plane Coordinate Systems

In both Montana and Wyoming, scale factors are used to compute grid distances from measured ground distances. In Montana, 'Combination Scale Factors' are the product of the specific scale factor (a factor based on local latitude used to compute the difference between the ellipsoid and grid distance) and the elevation scale factor (a factor based on project elevation used to compute the difference between ground distance and ellipsoid distance). In Wyoming, 'Datum Adjustment Factors' are computed in the same manner, by multiplying a grid scale factor by an elevation scale factor.

Traditionally these factors were determined from tables<sup>(14)</sup>. Later with the advent of NAVD 88 and computer geodesy programs the 'height above the ellipsoid' was used in place of the elevation above sea level. Essentially, project Scale Factors were divided into the State Plane northing and easting coordinate values of the project control points, thereby scaling the values of the control points to yield LDPC coordinates. This method allows for the LDPC grid measurements to closely match actual ground

distances measured and the project basis of bearing still remains the same as the State Plane grid. While this system generally works well, there are some inherent problems with this system:

- LDPC systems represent only low distortion areas (i.e., in general does not minimize distortion over as large an area as can be achieved using a customized projection)
- LDPC coordinates look similar to state plane coordinates, but are NOT
- As a scaled version of a true map projection, it cannot be geo-referenced (requires reversion calculation back to State Plane Coordinates)
- Each project is on a unique stand alone LDPC system
- Not directly compatible with any recognized datum or the National Spatial Reference System (NSRS).

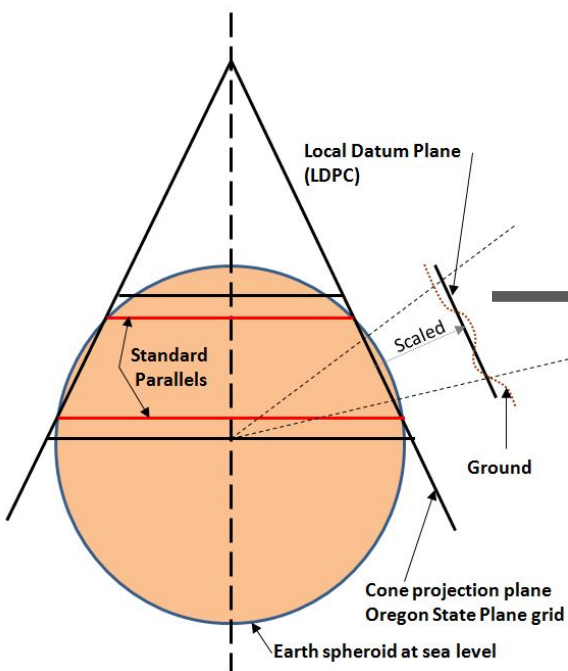


Figure 1.7: Local Datum Plane Coordinate System scaled from State Plane [mla]

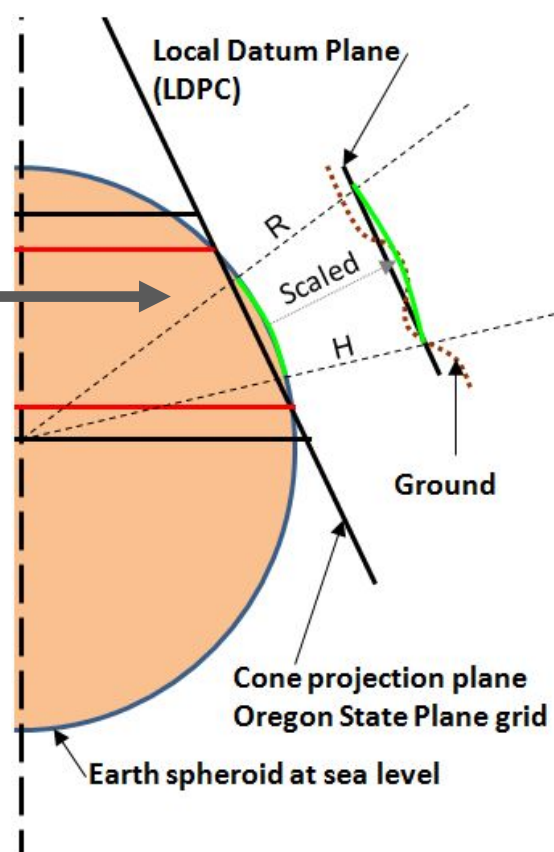


Figure 1.7.1: Local Datum Plane Coordinate System enlarged to show spheroid to LDPC plane

### 1.5.2 Low Distortion Map Projection Systems

Low distortion map projections (like those within the RMTCRS coordinate system) are based on true conformal projections designed to cover specific portions of urban and rural areas of the state. For conformal projections (e.g., Transverse Mercator, Lambert Conformal Conic, Stereographic, Oblique Mercator (RSO), regular Mercator, etc.), linear distortion is the same in every direction from a point. That is, the scale at any particular point is the same in any direction and figures on the surface of the Earth tend to retain their original form on the map. In addition, angles on the Earth are the same as on

the map. The term 'low distortion' refers to minimizing the lineal horizontal distortion from two affects: 1) representing a curved surface on a plane and 2) departure of the elevated topography from the projection surface due to variation in the regional height of the area covered. See Section 2.2 for more information on map projection distortion.

The advantages of a low distortion projection are:

- Grid coordinate zone distances very closely match the same distance measured on the ground
- Allow for larger areas (than LDPC) to be covered with less distortion
- Reduced convergence angle (if the central meridian is centered within the zone)
- Quantitative distortion levels can be determined from topographic heights
- Clean zone parameter definitions compatible with common surveying, engineering, and GIS software
- Easy to transform between other coordinate systems
- Maintains a relationship to the National Spatial Reference System (NSRS) by allowing direct use of published NSRS control coordinates (i.e., latitude, longitude, and ellipsoid height)
- Can cover entire cities and counties making them useful for regional mapping and GIS

### **1.5.3 Projection Grid Coordinates**

Because calculations relating latitude and longitude to positions of points on a given map can become quite involved, rectangular grids have been developed for the use of surveyors, engineers, and GIS mapping professionals. In this way, each point may be designated merely by its distance from two perpendicular axes on the 'plane' map. The 'Y' axis normally coincides with a chosen central meridian, 'y' increasing north. The 'X' axis is perpendicular to the 'Y' axis at a latitude of origin on the central meridian, with 'x' increasing east. Commonly, 'x' and 'y' coordinates are called "eastings" and "northings," respectively, and to avoid negative coordinates may have "false eastings" and "false northings" added to relate to the projection grid origin.



## Chapter 2 Coordinate System Geodesy

### 2.1 Types of Conformal Map Projections Used for the RMTCRS

#### 2.1.1 Lambert Conformal Conic Projection

The Lambert Conformal Conic projection (created in 1772 by Johann Heinrich Lambert), is one of the most commonly used low distortion projections and was used for the Montana State Plane Coordinate System. As the name implies, the Lambert projection is conformal (preserves angles with a unique scale at each point). This projection superimposes a cone over the sphere of the Earth, with either one reference parallel tangent (or above the globe in the case of a low distortion projection) or with two standard parallels secant (a straight line that intersects with the globe in two places). Specifying a 'central meridian' orients the cone with respect to the ellipsoid. Scale error (distortion with respect to the ellipsoid) is constant along the parallel(s). Typically, it is best used for covering areas long in the east–west direction, or, for low distortion applications, where topographic height changes more-or-less uniformly in the north–south direction. The Lambert Conformal Conic projection for relatively large regions is designed as a single parallel Lambert projection. The cone of the projection is typically scaled up from the ellipsoid to 'best fit' an area and range of topographic height on the Earth's surface (see Figure 2.2.3).

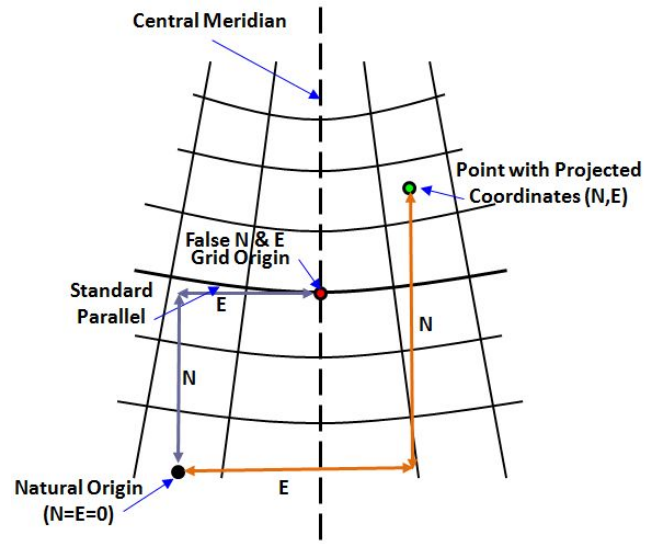


Figure 2.1.1: Diagram for Lambert Conical Conformal Projection with one standard parallel

The Lambert Conformal Conic projection for relatively large regions is designed as a single parallel Lambert projection. The cone of the projection is typically scaled up from the ellipsoid to 'best fit' an area and range of topographic height on the Earth's surface (see Figure 2.2.3).

#### 2.1.2 Transverse Mercator Projection

The Transverse Mercator (ellipsoidal) map projection was originally presented by mathematician Carl Friedrich Gauss in 1822. It is a conformal projection that is characterized by a cylinder superimposed over the ellipsoid of the earth with a straight central meridian. Distances along the meridian have a constant scale. This projection is used for the familiar UTM (Universal Transverse Mercator) map projection series, and it is the most commonly used in geodetic mapping especially for areas of study that are relatively close to the central meridian. This project works particularly well for areas long in the north – south direction, and for low distortion applications where topographic height changes more-or-less uniformly in the east-west direction. This projection was used for the Wyoming State Plane Coordinate System.

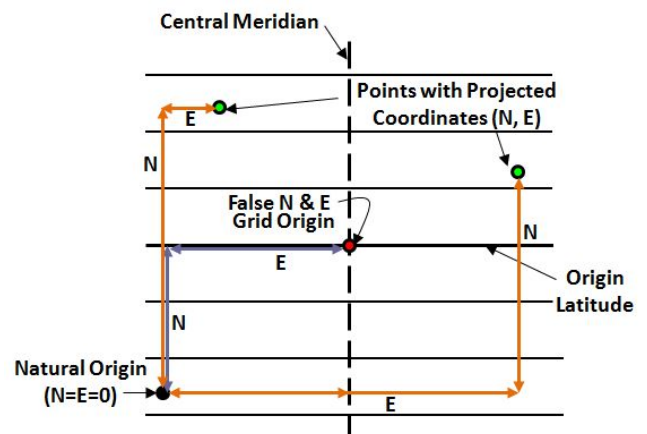


Figure 2.1.2: Diagram Transverse Mercator Projection [m1a]

### 2.1.3 Oblique Mercator (RSO) Projection

Although not used for the RMTCRS, various forms of the Oblique Mercator (OM) projection have been developed, and the ellipsoidal form used for was published by Martin Hotine in 1947<sup>(8)</sup>. Hotine called it the Rectified Skew Orthomorphic (RSO) projection, and it still goes by this name in some publications and software. It is an oblique form (rotated cylinder) of the Mercator conformal map projection. The 'Initial Line' is the centerline (projection skew axis) and is specified with one point and an azimuth (or skew angle) which may be positive or negative (right or left). This projection is typically used for long linear features that run at 'angle' to what would otherwise be normal north-south or east-west conventions. Here the projection centerline is along a geodesic, at an oblique angle (rotated cylinder), and the process is to specify the projection local origin latitude and longitude together with the centerline (Initial Line) azimuth to be the line that runs parallel and centered near the alignment of the key object or landform such as a coast line, river, or island chain feature of the Earth. Along this Initial Line the scale is true (one) much like the normal Mercator projection and perpendicular from this line the scale varies from one. This projection works well when the areas of study are relatively close to this line. The specified 'grid origin' is located where north and east axes are zero. In contrast, the 'natural origin' of the projected coordinates is located where the 'Initial Line' of the projection crosses the 'equator of the aposphere' (a surface of constant total curvature), which is near (but not coincident with) the ellipsoid equator (see Figure 2.1.1). The ellipsoid is conformally mapped onto the aposphere, and then to a cylinder, which ensures that the projection is strictly conformal. However, unlike the TM projection, where the scale is constant along the central meridian, the scale (with respect to the ellipsoid) is not quite constant along the Initial Line (rather it is constant with respect to the aposphere). But the variation in scale along the Initial Line is small for large areas. Note that this projection can also be defined by specifying the Initial Line using two points or with a single point and a skew azimuth.

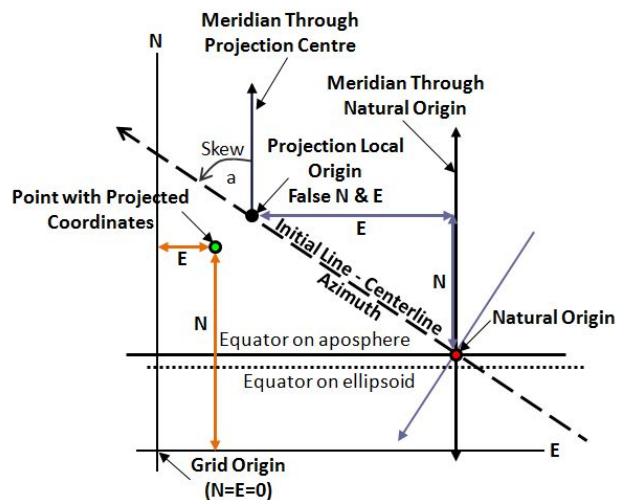


Figure 2.1.3: Diagram for Oblique Mercator (RSO) Projection *lmlal*

## 2.2 Managing Map Projection Distortion

### 2.2.1 Distortion is Unavoidable

Johann Carl Friedrich Gauss's (1777–1855) Theorema Egregium (Remarkable Theorem) mathematically proved that a curved surface (such as the Earth's ellipsoid model) cannot be represented on a plane without distortion. Since any method of representing a sphere's surface on a plane is a map projection, all map projections produce distortion and every distinct map projection distorts in a distinct way. For low distortion projections, deciding on the type of map projection in order to minimize the distortion for an area of the earth may not be an obvious or clear-cut task.

### 2.2.2 Two General Types of Map Projection Distortion by Michael L. Dennis, PE, RLS

**1. Linear distortion** - The difference in distance between a pair of grid (map) coordinates when compared to the true (ground) distance is shown by  $\delta$  in tables 2.2.2.1 and 2.2.2.2. This may be

expressed as a ratio of distortion length to ground length: E.g., feet of distortion per mile; parts per million (= mm per km). *Note:* 1 foot / mile = 189 ppm = 189 mm / km.

**Linear distortion can be positive or negative:**

Negative distortion means the grid (map) length is shorter than the “true” horizontal (ground) length.

Positive distortion means the grid (map) length is longer than the “true” horizontal (ground) length.

(continued on next page)

## Linear distortion due to Earth curvature

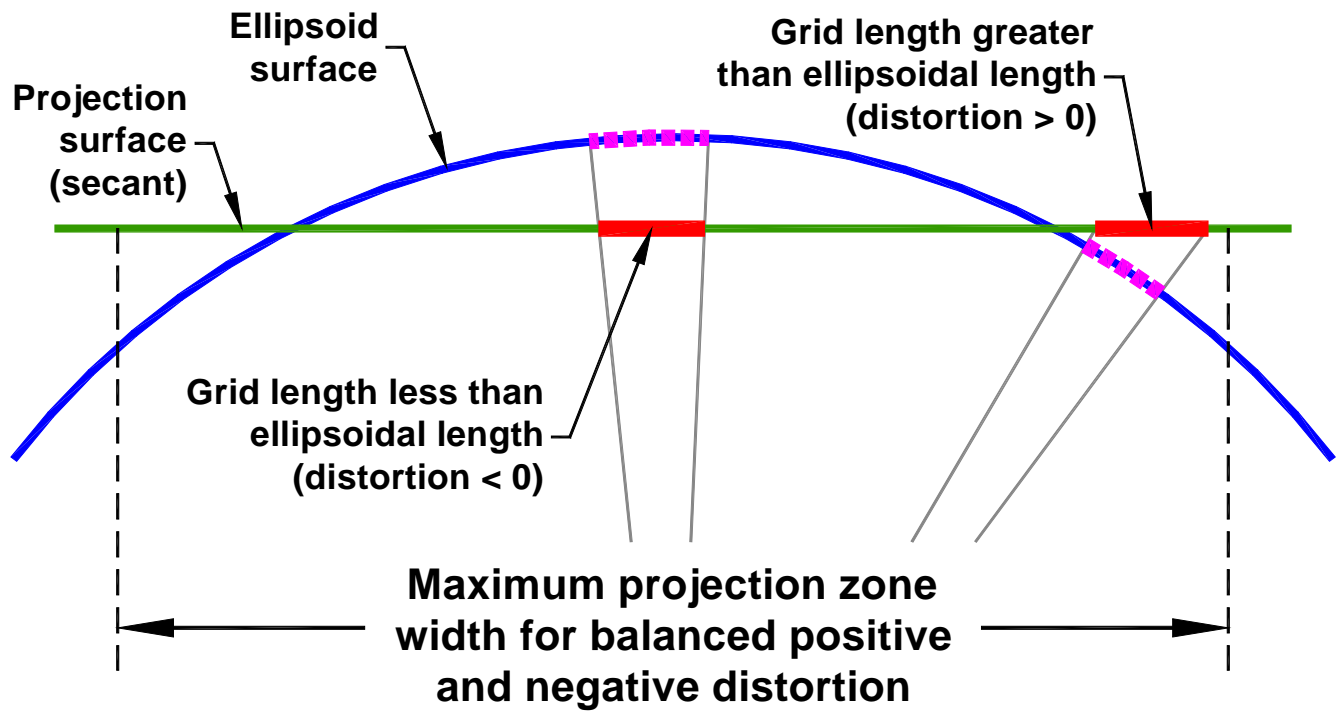


Table 2.2.2.1

Maximum zone width for secant projections (km and miles)	Maximum linear horizontal distortion, $\delta$		
	Parts per million (mm/km)	Feet per mile	Ratio (absolute value)
25 km (16 miles)	$\pm 1$ ppm	$\pm 0.005$ ft/mile	1 : 1,000,000
57 km (35 miles)	$\pm 5$ ppm	$\pm 0.026$ ft/mile	1 : 200,000
81 km (50 miles)	$\pm 10$ ppm	$\pm 0.05$ ft/mile	1 : 100,000
114 km (71 miles)	$\pm 20$ ppm	$\pm 0.1$ ft/mile	1 : 50,000
180 km (112 miles)	$\pm 50$ ppm	$\pm 0.3$ ft/mile	1 : 20,000
255 km (158 miles) e.g., SPCS*	$\pm 100$ ppm	$\pm 0.5$ ft/mile	1 : 10,000
510 km (317 miles) e.g., UTM <sup>†</sup>	$\pm 400$ ppm	$\pm 2.1$ ft/mile	1 : 2,500

\*State Plane Coordinate System; zone width shown is valid between  $\sim 0^\circ$  and  $45^\circ$  latitude

<sup>†</sup>Universal Transverse Mercator; zone width shown is valid between  $\sim 30^\circ$  and  $60^\circ$  latitude

## Linear distortion due to ground height above ellipsoid

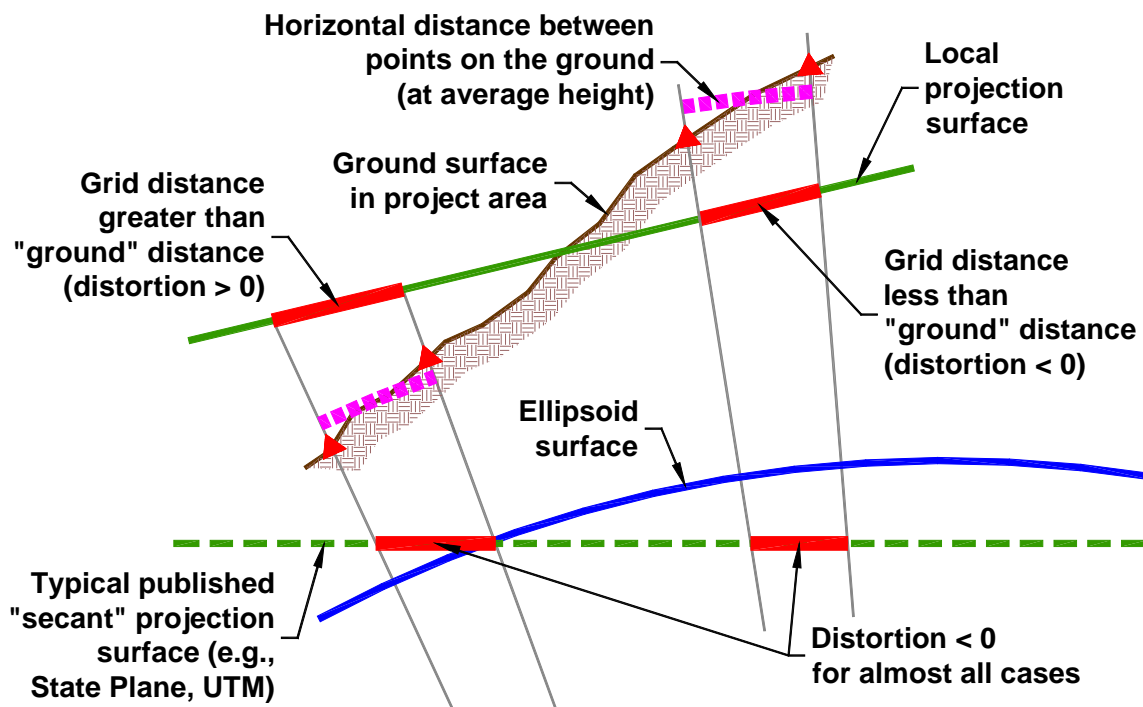


Table 2.2.2.2

Height below (-) and above (+) projection surface	Maximum linear horizontal distortion, $\delta$		
	Parts per million (mm/km)	Feet per mile	Ratio (absolute value)
$\pm 30$ m ( $\pm 100$ ft)	$\pm 4.8$ ppm	$\pm 0.025$ ft/mile	$\sim 1 : 209,000$
$\pm 120$ m ( $\pm 400$ ft)	$\pm 19$ ppm	$\pm 0.10$ ft/mile	$\sim 1 : 52,000$
$\pm 300$ m ( $\pm 1000$ ft)	$\pm 48$ ppm	$\pm 0.25$ ft/mile	$\sim 1 : 21,000$
+600 m (+2000 ft)*	-96 ppm	-0.50 ft/mile	$\sim 1 : 10,500$
+1000 m (+3300 ft)**	-158 ppm	-0.83 ft/mile	$\sim 1 : 6,300$
+4400 m (+14,400 ft) <sup>†</sup>	-688 ppm	-3.6 ft/mile	$\sim 1 : 1,500$

\* Approximate mean topographic height of North America (US, Canada, and Central America)

\*\* Approximate mean topographic height of western coterminous US (west of 100°W longitude)

<sup>†</sup> Approximate maximum topographic height in coterminous US

### **Rule of Thumb:**

A 30 m (100-ft) change in height causes a 4.8 ppm change in distortion

Creating an LDP and minimizing distortion by the methods described in this document only makes sense for conformal projections. For conformal projections (e.g., Transverse Mercator, Lambert Conformal Conic, Stereographic, Oblique Mercator (RSO), regular Mercator, etc.), linear distortion is the same in every direction from a point. For all non-conformal projections (such as equal area projections), linear distortion generally varies with direction, so there is no single unique linear distortion (or “scale”) at any point.

**2. Angular distortion** - For conformal projections (e.g., Transverse Mercator, Lambert Conformal Conic, Stereographic, Oblique Mercator, etc.), this equals the *convergence (mapping) angle* ( $\gamma$ ). The convergence angle is the difference between grid (map) north and true (geodetic) north. Convergence angle is zero on the projection central meridian, positive east of the central meridian, and negative west of the central meridian as shown in table 2.2.2.3 below.

The magnitude of the convergence angle increases with distance from the central meridian, and its rate of change increases with increasing latitude.

Table 2.2.2.3 shows ‘convergence angles’ at a distance of one mile (1.6 km) east (positive) and west (negative) of projection central meridian (for both Transverse Mercator and Lambert Conformal Conic projections).

Table 2.2.2.3

Latitude	Convergence angle 1 mile from CM	Latitude	Convergence angle 1 mile from CM
0°	0° 00' 00"	50°	±0° 01' 02"
10°	±0° 00' 09"	60°	±0° 01' 30"
20°	±0° 00' 19"	70°	±0° 02' 23"
30°	±0° 00' 30"	80°	±0° 04' 54"
40°	±0° 00' 44"	89°	±0° 49' 32"

Usually convergence is not as much of a concern as linear distortion, and it can only be minimized by staying close to the projection central meridian (or limiting surveying and mapping activities to equatorial regions of the Earth). Note that the convergence angle is zero for the regular Mercator projection, but this projection is not suitable for large-scale mapping in non-equatorial regions. In many areas, distortion due to variation in ground height is greater than that due to curvature. **The total linear distortion of grid (map) coordinates is a combination of distortion due to Earth curvature and distortion due to ground height above the ellipsoid.**

### 2.2.3 Six Steps for Designing a Low Distortion Projection (LDP) by Michael L. Dennis, PE, RLS

#### Step 1. Define the project area and choose a representative ellipsoid height, $h_o$ (not elevation)

The average height of an area may not be appropriate (e.g., for projects near a mountain). Usually there is no need to estimate height to an accuracy of better than about  $\pm 6$  m ( $\pm 20$  ft). Note that as the size of the area increases, the effect of Earth curvature on distortion increases, and it must be considered in addition to the effect of topographic height, E.g., for areas wider than about 56 km (35 miles) perpendicular to the projection axis (i.e.,  $\sim 28$  km or  $\sim 18$  miles either side of projection axis), distortion due to curvature alone exceeds 5 parts per million (ppm). The “projection axis” is defined in step #2.

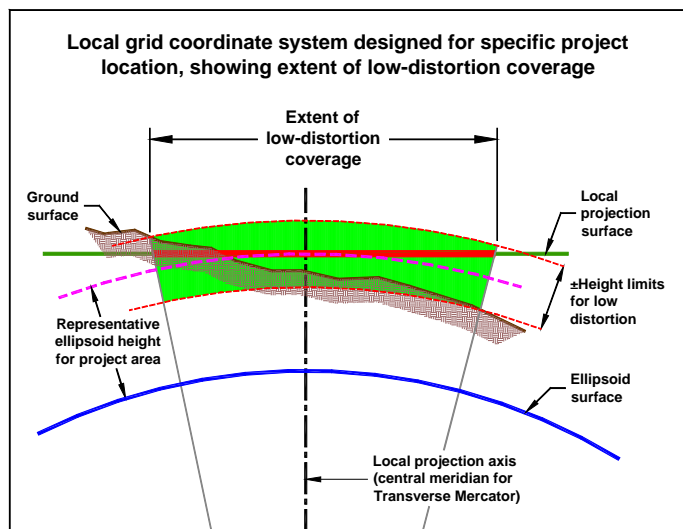


Figure 2.2.3: Diagram shows the effect of scaling the projection to a representative height above the ellipsoid [md]

#### Step 2. Choose the projection type and place the projection axis near the centroid of the project area.

Select a well-known and widely used conformal projection, such as the Transverse Mercator (TM), one-parallel Lambert Conformal Conic (LCC), or Oblique Mercator (OM/RSO).

When minimizing distortion, it will not always be obvious which projection type to use, but for small areas ( $< \sim 55$  km or  $\sim 35$  miles wide perpendicular to the projection axis), usually both the TM and LCC will provide satisfactory results.

When in doubt, the TM is a good choice for most applications, since it is probably the map projection supported across the broadest range of software packages. However, commercial software vendors are adding more user-definable projections, and so over time the problem of projection availability should diminish.

In nearly all cases, a two-parallel LCC should *not* be used for an LDP with the NAD 83 datum definition (but note that some software may not support a one-parallel LCC). A two-parallel LCC should not be used because the reason there are two parallels is to make the projection secant to the ellipsoid (i.e., the central parallel scale is less than 1). This is at odds with the usual objective of scaling the projection so that the developable surface is at the topographic surface, which is typically above the ellipsoid, particularly in areas where reduction in distortion is desired.

The OM (RSO) projection can be very useful for minimizing distortion over large areas, especially areas that are more than about 56 km (35 miles) long in an oblique direction. It can also be useful in areas where the topographic slope varies gradually and more-or-less uniformly in a direction other than north-south or east-west. The disadvantage of this projection is that it is more difficult to evaluate, since another parameter must be optimized (the projection skew axis). In addition, this projection is more complex, and may not be available in as many software packages as the TM and LCC.

The Oblique Stereographic (OS) projection can also provide satisfactory results for small areas, but it has the disadvantage of not conforming to Earth curvature in any direction. In situations where this projection works well, there really is no reason to use it, because the TM projection will give equally good (if not better) results. In very rare cases this projection might give the best results, such as bowl-shaped areas.

Bear in mind that universal commercial software support is not an essential requirement for selecting a projection. In the rare cases where third parties must use a coordinate system based on a projection not supported in their software, it is always possible for them to get on the coordinate system implicitly (i.e., by using a best-fit procedure based on coordinate values).

The 'projection axis' is the line along which projection scale is constant (with respect to the ellipsoid). It is the central meridian for the TM projection, the standard (central) parallel for the one-parallel LCC projection, the (implicitly defined) central parallel for the two-parallel LCC projection, and the skew axis for the OM projection (actually the scale is not quite constant along the skew axis, as discussed in Section 2.1.3). The OS projection does not have a projection axis (projection scale is only constant at one point).

Place the central meridian of the projection near the east-west “middle” of the project area in order to minimize convergence angles (i.e., the difference between geodetic and grid north).

In some cases it may be advantageous to offset the projection axis from project centroid (e.g., if topographic height increases or decreases gradually and more-or-less uniformly perpendicular to the projection axis).

### Step 3. Scale the central meridian of the projection to representative ground height, $h_0$

Compute map projection axis scale factor “at ground”:  $k_0 = 1 + \frac{h_0}{R_G}$

For the TM projection,  $k_0$  is the central meridian scale factor.

For the one-parallel LCC projection,  $k_0$  is the standard (central) parallel scale factor.

For the OM projection,  $k_0$  is the projection skew axis scale at the local origin.

For the OS projection,  $k_0$  is the scale at the projection origin.

$R_G$  is the geometric mean radius of curvature,  $R_G = \frac{a\sqrt{1-e^2}}{1-e^2 \sin^2 \varphi}$

and  $\varphi$  = geodetic latitude of point, and for the GRS-80 ellipsoid:

$$a = \text{semi-major axis} = 6,378,137 \text{ m (exact)} \quad = 20,925,646.325 \text{ international ft.}$$

$$= 20,925,604.474 \text{ US survey ft.}$$

$$e^2 = \text{first eccentricity squared} = 2f - f^2$$

$$f = \text{geometric flattening} = 1 / 298.257222101$$

Alternatively, can initially approximate  $R_G$  since  $k_0$  will likely be refined in Step #4, by using  $R_G$  values in Table 2.2.3.1.



Geometric mean radius of curvature at various latitudes for the GRS-80 ellipsoid (rounded to nearest 1000 meters and feet).

Table 2.2.3.1

Latitude	$R_G$ (meters)	$R_G$ (feet)	Latitude	$R_G$ (meters)	$R_G$ (feet)
0°	6,357,000	20,855,000	50°	6,382,000	20,938,000
10°	6,358,000	20,860,000	60°	6,389,000	20,961,000
20°	6,362,000	20,872,000	70°	6,395,000	20,980,000
30°	6,367,000	20,890,000	80°	6,398,000	20,992,000
40°	6,374,000	20,913,000	90°	6,400,000	20,996,000

#### Step 4. Check the distortion at points distributed throughout project area

The best approach here is to compute distortion over entire area and generate distortion contours (this ensures optimal low-distortion coverage). This may require repeated evaluation using different  $k_0$  values. It may also warrant trying different projection axis locations and different projection types.

Distortion computed at a point (at ellipsoid height  $h$ ) as  $\delta = k \left( \frac{R_G}{R_G + h} \right) - 1$

Where  $k$  = projection grid point scale factor (i.e. “distortion” with respect to the ellipsoid at a specific point). Note that computation of  $k$  is rather involved, and is often done by commercially available software. However, if your software does not compute  $k$ , or if you want to check the accuracy of  $k$  computed by your software, equations for doing so for the TM and LCC projections are provided later in this document. Because  $\delta$  is a small number for low distortion projections, it is helpful to multiply  $\delta$  by 1,000,000 to express distortion in parts per million (ppm).

#### Step 5. Keep the definition simple and clean

Define  $k_0$  to no more than six decimal places, e.g., 1.000206 (exact). *Note:* A change of one unit in the sixth decimal place equals distortion caused by a 6.4-meter (21-foot) change in height. Defining central meridian and latitude of grid origin to nearest whole arc-minute is usually adequate (e.g., central meridian = 111°48’00” W).

Define grid origin using whole values with as few digits as possible (e.g., false easting = 50,000 for a system with maximum easting coordinate value < 100,000). Note that the grid origin definition has no effect whatsoever on the map projection distortion.

It is strongly recommended that the coordinate values everywhere in the design area be distinct from other coordinate system values for that area (such as State Plane or UTM) in order to reduce the risk of confusing the LDP with other systems. *Note:* In some applications, there may be an advantage to using other criteria for defining the grid origin. For example, it may be desirable for all coordinates in the design area to have the same number of digits (such as six digits, i.e., between 100,000 and 999,999). In other cases it may be useful to make the coordinates distinct from State Plane by using larger rather than smaller coordinates, especially if the LDP covers a very large area.

#### Step 6. Explicitly define linear unit and geometric reference system (i.e., geodetic datum)

E.g., Linear unit = metric; (or) Linear unit = international foot; Geometric reference system = NAD 83 (2007).

The international foot is shorter than the US survey foot by 2 ppm. Because coordinate systems typically use large values, it is critical that the type of foot used be identified (the values differ by 1 foot

per 500,000 feet). *Note:* The reference system realization (i.e., “datum tag”) is not an essential component of the coordinate system definition. However, the datum tag is an essential component for defining the spatial data used within the coordinate system. This is shown in a metadata example later in this document. For NAD 83, the NGS convention is to give the datum tag in parentheses after the datum name, usually as the year in which the datum was “realized” as part of a network adjustment. Common datum tags are listed below:

- “2011” for the NSRS2011 (National Spatial Reference System of 2011) realization.
- “2007” for the NSRS2007 (National Spatial Reference System of 2007) realization.
- “199x” for the various HARN (or HPGN) realizations, where x is the last digit of the year of the adjustment (usually done for a particular state). In Montana and Wyoming a HARN/HPGN adjustment was done in 1992, so its datum tag is “1992”(there was also a readjustment performed in 1999 with a corresponding “1999” datum tag). The HARN and HPGN abbreviations are equivalent, and they stand for “High Accuracy Reference Network” and “High Precision Geodetic Network”, respectively.
- “CORS” for the realization based on the CORS network, and currently corresponding to 2002.00 for the coterminous US and Hawaii (and 2003.00 in Alaska).
- “1986” for the original NAD 83 realization. Because of the coordinate changes that occurred as part of the HARN/HPGN and NSRS2007 readjustments, this realization is not appropriate for data with horizontal accuracies of better than about 1 meter.

## 2.3 What Constitutes a Complete Coordinate System?

A complete 3D coordinate system is made up of a combination of horizontal and vertical datum, a geoid model, and a map projection definition. Each of these has certain aspects to consider which are briefly discussed below.

### 2.3.1 Ellipsoid Models

The overall shape of the earth is modeled by an ellipsoid of revolution (sometimes referred to as a spheroid). In order to imagine an ellipsoid model for the earth, align the shorter axis with the polar axis of the earth. Centrifugal force caused by the earth’s rotation creates a ‘squash’ effect where the radius of the earth is greater at the equator. The shape of the ellipsoid representing the earth is defined by mathematical models. Defining the latitude and longitude of particular points on the earth defines the origin and orientation of the ellipsoid. The North American Datum of 1983 (NAD 83) uses an ellipsoid model called the Geodetic Reference System of 1980 (GRS-80), which is very similar to the World Geodetic System of 1984 (WGS-84) ellipsoid. WGS-84, was created about the same time by the US Department of Defense. The WGS-84 datum definition continues to be minutely refined over time (although the WGS-84 ellipsoid definition remains fixed). Table 2.3.1 shows how similar GRS-80 is to WGS-84 in metric units, (note that the two numbers completely define the ellipsoid dimensions, and typical convention is to define the ellipsoid with the semi-major axis and reciprocal flattening, which are used to compute the semi-minor axis).

Table 2.3.1

Ellipsoid Model	Semi-Major Axis (exact by definition)	Semi-Minor Axis (computed)	Reciprocal Flattening (exact by definition)
WGS-84	6 378 137	6 356 752.314245	298.257223563
GRS-80	6 378 137	6 356 752.314140	298.257222101

### 2.3.2 Datum Transformations (seven parameter)

Sometimes called the Helmert Transformation after Friedrich Robert Helmert (1843-1917), this seven parameter transformation is the typical (common) geodetic method for moving the coordinates of a point or group of points from one coordinate system referenced to one datum into coordinates referenced to a different datum for a given instant in time. For the purposes of this discussion, a (local) coordinate system contains the necessary elements to convert WGS-84 geodetic positions observed with GPS (GNSS) to a particular coordinate/datum realization. Each projection zone coordinate system may be based on the choice of a particular defined datum, adjustment, and epoch such as NAD 83(2011), NAD 83 (2007), NAD 83(CORS)Epoch2002 or other NAD 83 realizations (see software vendor choices). As previously described, the defined datum relies on an ellipsoid model such as GRS-80 (used for NAD 83 and the ITRS). These seven parameters account for the following:

**Translation X-** Translation along the X-axis

**Translation Y-** Translation along the Y-axis

**Translation Z-** Translation along the Z-axis

**Scale Factor**

**Rotation X-** Rotation about the X-axis

**Rotation Y-** Rotation about the Y-axis

**Rotation Z-** Rotation about the Z-axis

Transformation equations and parameters provide a means of transforming coordinates referenced to one datum into coordinates referenced to a different datum. In general, two three-dimensional coordinate systems in space are related to each other by the following equation for Cartesian coordinates:

$$[ X Y Z ] \text{ Datum 'A'} = [ \Delta X \Delta Y \Delta Z ] + ( 1 + \Delta S ) [ 1 -R_z R_y R_z 1 -R_x -R_y R_x 1 ] [ X Y Z ] \text{ Datum 'B'}$$

Where;

$\Delta X$ : Shift along x-axis

$\Delta Y$ : Shift along y-axis

$\Delta Z$ : Shift along z-axis

S: Scale factor

Rx: Rotation about x-axis

Ry: Rotation about y-axis

Rz: Rotation about z-axis

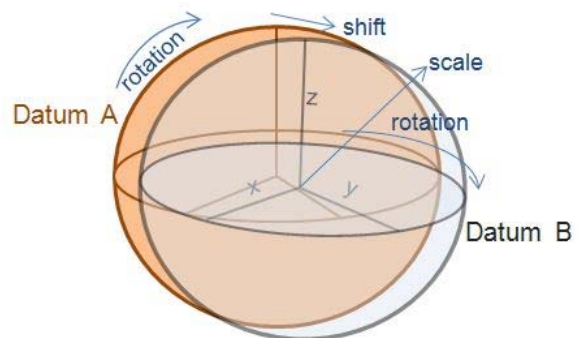


Fig. 2.3.2 [mla]

The first step is to know precisely the datum to which your input data are referenced. If your processing will require that this data be transformed to another coordinate system which is not based on the same datum, then you must consider the required datum transform. The following described example will consider the common case in which input data is referenced to WGS-84(G1150) and requires being converted to a coordinate system based on NAD 83(CORS96, 2007, or 2011), as these are the current versions of those datums. It is important to note here that for these particular datums, it will also be required to know the date to which the GPS data are processed, also known as the epoch of the data.

To consider a seven-parameter datum transform from WGS-84 to NAD 83, obtaining the required parameters for the Coordinate Frame datum transform is based on several assertions:

We can say that WGS-84(G1150) is equivalent to ITRF 00, the International Terrestrial Reference Frame of 2000, to an accuracy of approximately one centimeter<sup>(9)</sup>. Also, a 14-parameter (add time variables) transform has been defined between ITRF 00 and NAD 83(CORS96) and, for a given instant in time, the 14-parameter transformation may be represented as a 7-parameter coordinate frame transform. While no direct transforms have been defined from WGS-84(G1150) to NAD 83(CORS96), the transform from NAD 83(CORS96) is defined from ITRF 00 which creates the path through which the desired transform

can be completed. This 14-parameter transformation is specified in *“Transforming Position and Velocities between the International Terrestrial Reference Frame of 2000 and North American Datum of 1983”*, by Tomas Soler and Richard Snay<sup>(10)</sup>. Further discussion of 14-parameter transformations are beyond the scope of this document. For further discussion of this topic and tools for doing additional analysis, visit the NGS Horizontal Time-Dependent Positioning (HTDP) webpage: (<http://www.ngs.noaa.gov/TOOLS/Htdp/Htdp.shtml>) and the CORS Coordinates webpage (<http://www.ngs.noaa.gov/CORS/metadata1/>). Tools are available at this site for transforming data between the datums described here and several others. Velocities for positions can also be predicted here, as well as transformation of points on different datums to different epochs.

### 2.3.3 Horizontal Reference Datum

A reference datum is a mathematical model of a realized known and constant surface which is used to determine the location of points on the earth. There are a large number of commonly referenced datums in use in North America but two of the most common in use are WGS-84/ITRF, and NAD 83. The North American Datum of 1983 (NAD 83) is a common horizontal control datum for the United States, Canada, Mexico, and Central America, based on a (nearly) geocentric origin and the Geodetic Reference System 1980 (GRS-80) ellipsoid. Horizontal datums also have ‘realizations’ or a variation of a model reference frame primarily created from official network adjustments performed by the National Geodetic Survey. For example, NAD 83(1986) is significantly different than NAD 83(CORS96), but NAD 83(CORS96) usually only differs by a few centimeters from NAD 83(HARN/HPGN), and NAD 83(CORS) only differs from NAD 83(2007) in the western US (they are considered functionally the same elsewhere in the US). For the majority of Montana and Wyoming, the horizontal coordinate change from NAD83(2007) to NAD 83(2011) is 2 to 4 centimeters. Each of these is based on a particular adjustment (i.e., realization) of NAD 83. The suffix tag example ‘CORS96 and the epoch date of 2002 (Epoch 2002)’ refer to an upgrade of NAD 83 positions and velocities for all CORS sites, except those on the Pacific Islands and Alaska, so that they equal the transformed values, of the then computed, ITRF00 positions and velocities. Transforming from one adjustment datum to another will result in a coordinate position shift in your point positions.

NAD 83(1986) was officially (according to the National Geospatial Intelligence Agency (NGA) [http://earth-info.nga.mil/GandG/coordsys/datums/NATO\\_DT.pdf](http://earth-info.nga.mil/GandG/coordsys/datums/NATO_DT.pdf)) a ‘zero transform’ from WGS-84 although the earth center and parameters for the two datum are slightly different. This ‘zero transform’ is commonly accepted by software vendors. This effectively made NAD 83(1986) and WGS-84(original) identical, except for extremely small difference in ellipsoid shape (maximum difference of 0.1 mm at the poles). This was referred to as NAD 83 “CONUS” (code NAR-C), and the “CONUS” designation continues to be used in various commercial software packages (although it is not used by the NGS). At the time this relationship was defined (1987), the location of earth’s center of mass was only known to about  $\pm 2$  m, so these datums were considered the ‘same’, to within  $\pm 2$  m. Presently, the earth’s center of mass is known to the centimeter level, and it is recognized that current realizations of NAD 83 and WGS-84 actually differ by about 1-2 m (depending on location). This legacy ‘zero transform’ is still commonly used by commercial software vendors, even though it is not actually correct, which has become a persistent source of confusion. Part of this confusion stems from the fact that “WGS-84” is the name of the ellipsoid and the datum, which is not typical geodetic practice (e.g., both NAD 83 and ITRF use the GRS-80 ellipsoid). Also, software vendors may have slight variations in datum naming conventions, especially those programs developed in foreign countries.

Most GPS (GNSS) processing software packages contain a large list of the world’s datum from which to select. For the purposes of this document, users should generally accept (or seed) control values in the datum specified for the project or by contract specification (a notable exception is using current ITRF as

seed coordinates for baseline processing when using precise ephemerides). Where available, real-time GPS Networks currently send correctors referenced to the NAD 83(2011) Epoch2010.00 datum. In 2012 the NGS adopted new NAD 83 coordinates and velocities for all U.S. CORS that are located where NAD 83 is defined.

Datums identified only as NAD 83 or WGS-84 are not specific enough to clearly define the reference frame of geodetic data. Additional information is needed that defines the realization or version of a particular datum. In the case of NAD 83, a “datum tag” must be appended to the name, such as NAD 83(1986), NAD 83(CORS96), NAD 83(2007), or NAD83(2011); likewise for WGS-84: WGS-84(G1150), WGS-84(original), etc. NAD 83 (2011) and WGS-84(G1150) are the current versions of these systems. While NAD 83(1986) and WGS-84(original) were 'equivalent datums' (to within  $\pm 2$  m), this is not the case for NAD 83(2011) and WGS-84(G1150). A datum transform is required when transforming points between any projected or geographic coordinate systems based on these datums. For these particular datums, the magnitude of the difference is on the order of two meters.

The NGS has adopted a realization of NAD 83 called NAD 83(2011) that is based on new observations, but remains consistent with CORS observations. The NAD83 (2011) realization is not a new datum, but uses the same origin, scale, and orientation as the previous CORS realization. This realization *approximates* (but is not, and can never be, equivalent to) the more rigorously defined NAD 83(CORS96) realization in which Continuously Operating Reference Station (CORS) coordinates are distributed. NAD 83(2007) was created by adjusting GPS data collected during various campaign-style geodetic surveys performed between the mid-1980's through 2005. For this adjustment, NAD 83(CORS96) positional coordinates for ~700 CORS were held fixed (predominantly at the 2002.0 epoch for the stable North American plate, but 2007.0 in Alaska and western CONUS) to obtain consistent positional coordinates for the ~70,000 passive marks. Derived NAD 83(2007) positional coordinates should be consistent with corresponding NAD 83(CORS96) positional coordinates to within the accuracy of the GPS data used in the adjustment and the accuracy of the corrections applied to these data for systematic errors, such as refraction. In particular, there were no corrections made to the observations for vertical crustal motion when converting from the epoch of the GPS survey into the epoch of the adjustment, while the NAD 83(CORS96) coordinates do reflect motion in all three directions at CORS sites. For this reason alone, there can never be total equivalency between NAD 83(2007) and NAD 83(CORS96).

Control for the NAD 83(2011) adjustment was provided by the CORS. For all states except AZ, CA, OR, WA, NV and AK, the values used were the NAD 83 epoch 2002.0 values currently published by NGS. For AZ, OR, WA, NV and AK, HTDP (version 2.9) was used to convert the currently published NAD 83 positions of the CORS to epoch 2007.0. Typically, for all stations on the stable North American plate, an epoch date will be shown – as is currently the practice on datasheets (subject to change). For the other states, an epoch date of 2007.0 will be shown. In those states, except CA, HTDP can be used with the currently published CORS to determine the proper value to use. In CA, the values as currently published on the CSRC website should be used to maintain consistency with NAD 83(2007).

#### **2.3.4 Vertical Reference Datum**

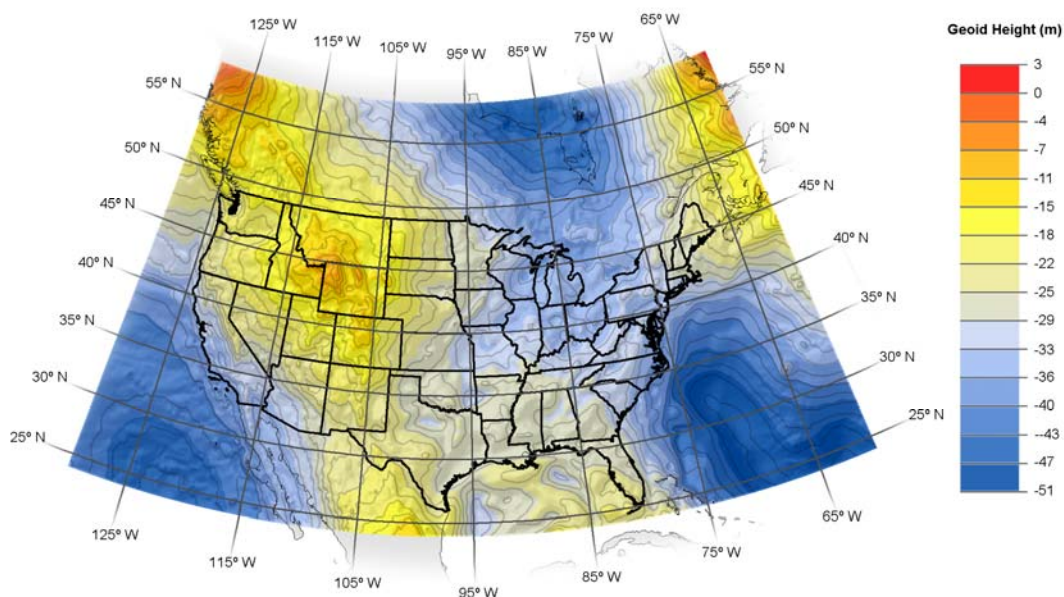
The North American Vertical Datum of 1988 (NAVD 88) was established in 1991 from a simultaneous, least squares, minimum constraint adjustment of Canadian, Mexican and United States leveling observations. It held fixed, the height of the primary tidal bench mark, named 'Father Point' in Rimouski, Quebec, Canada. Additional tidal bench mark elevations were not held due to the demonstrated variations in sea surface topography, i.e., the fact that mean sea level (as recorded by tide gages) is not a gravitational equipotential surface. NAVD 88 replaces NGVD 29 as the national standard geodetic reference for heights and is the only current vertical datum that works seamlessly with GPS (GNSS)

observation measurements and NAD 83. For more information on vertical datums see the NGS website <http://www.ngs.noaa.gov/faq.shtml#WhatVD29VD88>.

### 2.3.5 Geoid Models

A geoid [hybrid geoid model i.e., currently GEOID12A used in geodetic adjustments is comprised of a gravimetric scientific model constrained to a ‘best fit’ of a current benchmark monumented network (currently GPSBM2012). This hybrid model is updated by the National Geodetic Survey (NGS) approximately every three to six years as more gravity and bench mark data becomes available, and as new computational methods are developed. When measuring coordinates with GPS (GNSS) equipment within a project and coordinate system a geoid model such as GEOID12A must be applied (geoid height ‘N’) to allow for the conversion of measured NAD 83 ellipsoid heights (h) to orthometric heights (H) [equation  $H=h-(N)$ ] in the vertical datum NAVD88. The NGS 10 year plan outlines a transition to a pure gravimetric geoid model (GRAV-D) and new vertical datum by 2022.

Figure 2.3.5 GEOID12A Model Heights



See: <http://www.ngs.noaa.gov/GRAV-D/>

For Montana and Wyoming, the GEOID12A hybrid model is currently used. The GEOID99, 03, 06, and GEOID09(Conus) model were built with observation data and are no longer considered consistent with the physical earth. The GEOID12A model coverage over Montana and Wyoming includes additional satellite gravity data based on the new global geopotential model (EGM08) but otherwise varies from GEOID09 (Conus) in the following ways:

- Difference in ellipsoid heights (h) due to NGS’s National Adjustment of 2011.
- Difference in control data sets available at the time of generation.
- An additional signal (GOCO02S) was incorporated in the 2012 model, providing for more accurate and consistent terrain models.

The choice of geoid model is generally available in your GNSS vendor survey, engineering or GIS software and also within the National Geodetic Survey Online Positioning User Service (OPUS) program (<http://www.ngs.noaa.gov/OPUS/> under the Options menu).

### 2.3.6 RMTCRS Map Projection Parameter Units

As part of the ‘best practices’ approach to the creation of these zones, all of the RMTCRS map projection parameters are provided in metric units. Careful attention is needed when entering these map projection coordinate systems into the coordinate system management section of your GPS (GNSS) surveying, engineering, or GIS vendor software. When converting the provided metric data (false northing, false easting, etc.) to international or US feet, be sure to carry out the values to full sufficient significant figures (at least six decimal places) and check that the units are accepted by the software in the units you provide. Each software vendor (in the future) may elect to provide updated versions of their coordinate system management software with the RMTCRS zones already installed. Until that time it is recommended that you enter the projection parameters in metric units. Assigning units for a particular project, is a separate issue, and you may elect to choose English units of International Feet. Note that Montana requires the use of the International Foot where Wyoming requires the use of the US Foot.

### 2.3.7 US Foot vs. International Foot

The Rocky Mountain Tribal Coordinate Reference System grids were created and are defined by metric units. However, to conform to conventional survey practices, the projections are converted to the US Foot or the International Foot depending on state legislation. Foot type selection has long been the subject of internal debate among the professional survey community and this section is provided to clarify conversion from metric to imperial units.

Although both the US Foot and International Foot have merits, it is important to remain consistent in the use of the selected foot system. Use of the US Foot versus the International Foot is irrelevant when establishing a new coordinate system if all parties use the same foot system. To reference an existing project to a RMTCRS, the existing project must be re-projected into a RMTCRS. Once projected in a RMTCRS, the units may be changed between US and International feet using the 2 ppm conversion factor described below. Each existing project would require the same re-projection process regardless of type of “Foot” used. What is paramount is the same “Foot” is used for the current RMTCRS. Below is information regarding US Foot and International Foot from the NGS website:

#### ***What are the official conversions used by NGS to convert 1) meters to inches, and 2) meters to feet?***

*First, remember this rule: There is only one meter, BUT, there are two types of feet.*

*The two types of feet are:*

#### **1. The U.S. Survey Foot**

*It is defined as: 1 meter = 39.37 inches.*

*If you divide 39.37 by 12 (12 inches per foot), you get the conversion factor: 1 meter = 3.280833333... U.S. Survey Feet.*

#### **2. The International Foot**

*It is defined as: 1 inch = 2.54 centimeters.*

*If you convert this to meters and feet, you get the conversion factor: 1 International Foot = 0.3048 meters.*

*These two conversion factors produce results that differ by 2 parts per million; hence for most practical work it does not make any difference to the average surveyor which one is used since they usually do not*

encounter distances this large. For example, converting a distance of 304,800 meters (about 1,000,000 feet) to feet using the two conversion factors, these are the results:

*304,800 meters = 999,998.000 U.S. Survey Feet*

*304,800 meters = 1,000,000.000 International Feet*

*A difference of 2 feet in 1 million feet.*

*NGS has always used meters in their computations, so this has not been an issue for us. However, the one place where NGS does use feet, and the numbers are large enough to make a difference, is in the publication of rectangular State Plane Coordinates (SPCs).*

*For most of the years surveying has been undertaken in the United States, surveyors have used the U.S. Survey Foot. (Note: Some surveying historians will mention that other types of linear measure, mostly of Spanish origin, was also used in the United States) In fact, NGS originally computed and published SPCs in U.S. Survey Feet for many years when the reference system was the North American Datum of 1927 (NAD 27). And the conversion formulas (latitude/longitude to SPCs) were developed to produce U.S. Survey Foot values. In fact, NGS never published NAD 27 SPCs in meters.*

*However, most other countries, and the engineering community in the United States, began using the International Foot as established by the National Bureau of Standards (NBS), now the National Institute of Standards and Technology (NIST).*

*To make the transition in the surveying community, in 1959 NBS published a Federal Register notice stating that the U.S. surveying community would convert to the International Foot the next time the National Coordinate Reference System was updated with revised values. That revision of coordinate values (i.e., latitudes and longitudes) was realized when the North American Datum of 1983 (NAD 83) was computed and published in 1986.*

*NGS began publishing SPCs in meters because going metric was the direction the Federal government was heading to be consistent in a global economy, AND, the change in the size of the SPCs values was a way to alert users that they were using a new horizontal datum. Also, the new conversion formula (latitude/longitude to SPCs) produced meters, not feet. However, the surveying community in various states still wanted SPCs in feet. NGS did not want to mandate which foot (U.S. Survey or International) a state should use. So, NGS left that decision to the individual states.*

*NGS does NOT have an "official" conversion factor. NGS works in meters ONLY. NGS only uses feet to publish SPCs, and those are converted from meters using the conversion factor as defined by the individual states who have requested that we publish SPCs in feet.*

*The only other instance where NGS publishes linear values in feet is for elevations, i.e., orthometric heights. All computations are still done in meters, but for publication purposes we convert meters to feet. That conversion is done using the U.S. Survey Foot conversion factor. We publish elevations in meters to the nearest millimeter (3 decimal places) and in feet to hundredths of a foot (2 decimal places). For elevations above 5,000 feet (1,524 meters), the conversion factor will change the foot value by one in the second place.*



### **2.3.8 Adding a Map Projection to a Coordinate System**

Finally, a map projection must be chosen so the results can be displayed on a projected plane in a defined grid (northing's and easting's). In order to derive common northing and easting coordinates, a false northing and false easting are paired with the projection origin (central meridian and origin latitude). The map projection parameters (RMTCRS) provide a scale factor (based in part on the topographic height above ellipsoid) to better represent the local ground elevation within the useful limits (best range) of the zone topography (see figure 2.2.3). This scaling helps to define a threshold range in parts per million (ppm) of how closely the grid vs. ground distance measurements should match one another. For example, if the choice is to fit a threshold of  $\pm 10$  parts per million ( $\pm 10$ ppm) then the desire is to maintain an accuracy ratio maximum of 1:100 000, which would be a ten-fold improvement over the State Plane Coordinate Systems (as much as  $\sim 1:10$  000 with respect to ellipsoid, and significantly greater distortion in high elevation areas).

## Chapter 3 RMTCRS Map Projection Zones

### 3.1 The Development of RMTCRS Projection Zones in the Rocky Mountain Tribal Areas

The development of each map RMTCRS projection zone involved a hands-on process by the Technical Development Team of interested stakeholders, together with the aid of Michael Dennis of Geodetic Analysis LLC, Pima Arizona. Mr. Dennis has created proprietary software to facilitate the visualization of low distortion map projection zones. Each zone was developed through a multi-step iterative process to derive the best result as determined by the Technical Team using the 'best practices' approach outlined in Chapter 1. Two additional low distortion reference systems in Montana have been developed for the Billings and Bobcat (Bozeman) areas by Mr. Dennis and Rich Jensen, PLS with Sanderson and Stewart, but are not part of the tribal mapping project. Additional zones may be created and added to this chapter as time goes on. If you work in a particular area of the state and no current zone covers that area, you may wish to discuss future plans for an additional zone for your work area. Please call and discuss your needs with Northern Engineering & Consulting in Billings, Montana.

#### 3.1.1 The RMTCRS Zone Catalog for the Rocky Mountain Tribal Area

Table 3.1.1

Zone Name*	Projection	Latitude of Grid Origin	Central Meridian	False Northing (m)	False Easting (m)	Scale (exact)
St. Mary's Valley (Blackfeet)	TM	48°30'00"N	112°30'00"W	0	150 000	1.000 160
Blackfeet Indian Reservation	TM	48°00'00"N	112°30'00"W	0	100 000	1.000 190
Fort Belknap Indian Reservation	LCC	48°30'00"N	108°30'00"W	150 000	200 000	1.000 120
Milk River	LCC	48°30'00"N	111°00'00"W	200 000	150 000	1.000145
Fort Peck Indian Reservation-Sioux	LCC	48°20'00"N	105°30'00"W	50 000	100 000	1.000 090
Fort Peck Indian Reservation-Assiniboine	LCC	48°20'00"N	105°30'00"W	100 000	200 000	1.000 120
Wind River Indian Reservation	TM	42°40'00"N	108°20'00"W	0	100 000	1.000 240
Crow Indian Reservation	TM	44°45'00"N	107°45'00"W	0	200 000	1.000 148
Billings	LCC	45°47'00"N	108°25'00"W	50 000	200 000	1.000 1515
Bobcat	LCC	46°15'00"N	111°15'00"W	100 000	100 000	1.000 185

TM = Transverse Mercator

LCC = Lambert Conformal Conic projection (single parallel)

\*All zones designed with an initial target distortion level of  $\pm 20$  ppm = 1:50 000 Ratio =  $\pm 0.10'$ /mile.

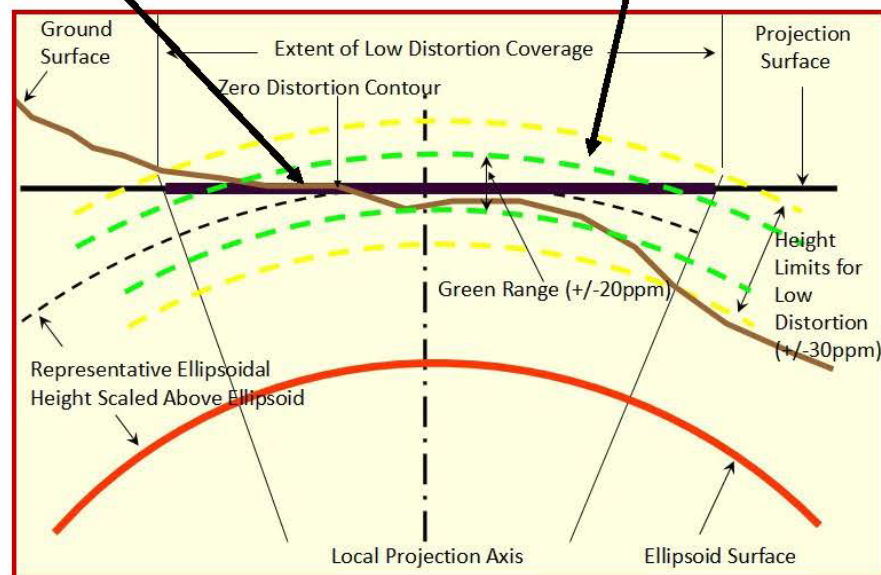
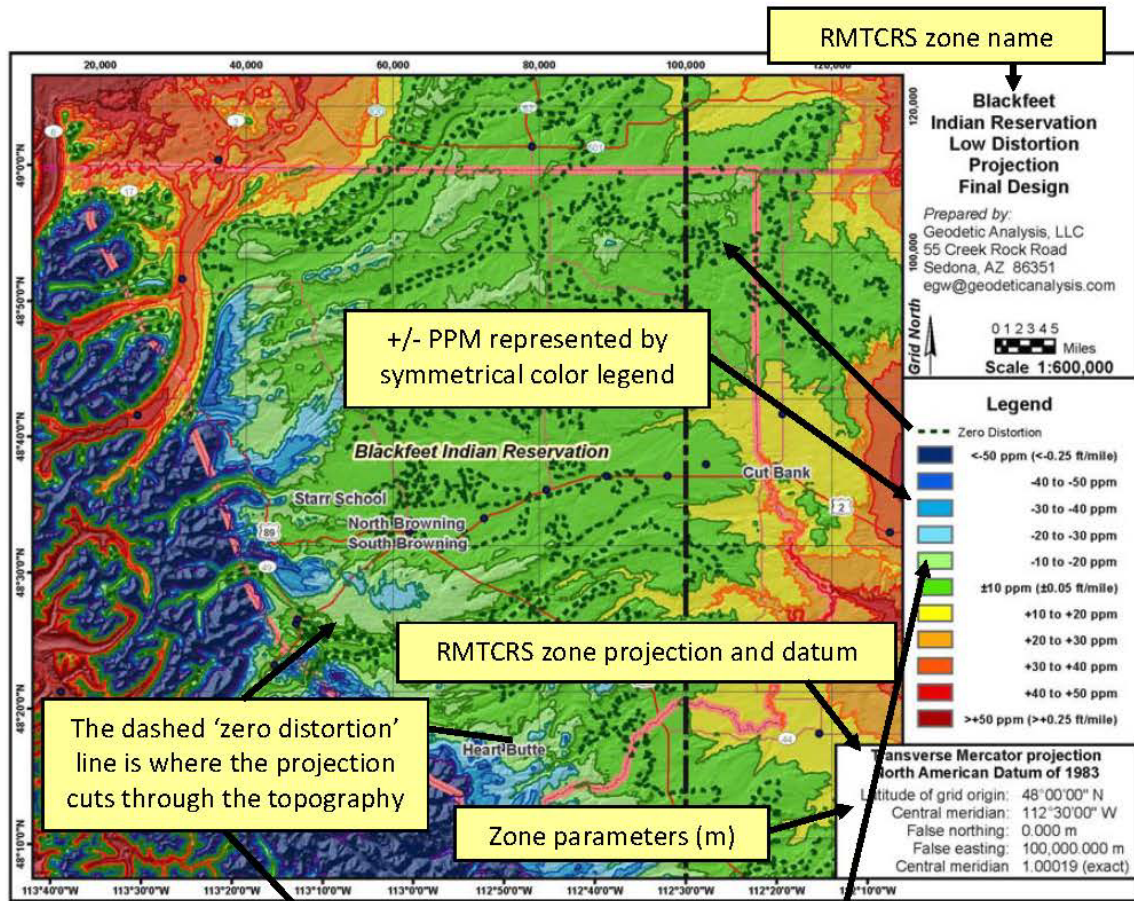
All lineal units are metric (m).

All zones reference the NAD 83 (2011) datum (Geometric Reference System)

*Refer to the RMTCRS map series shown in Appendix 'A', noting on each map the defined areas shown in green. These areas define the area where one can work within the  $\pm 10$  ppm or  $\pm 20$  ppm threshold as defined in the catalog above. As the ppm range increases the colors change accordingly as shown in the legend on each individual map.*

### 3.1.2 RMTCRS Zone Map Interpretation

Figure 3.1.2 : RMTCRS Zone Map Interpretation

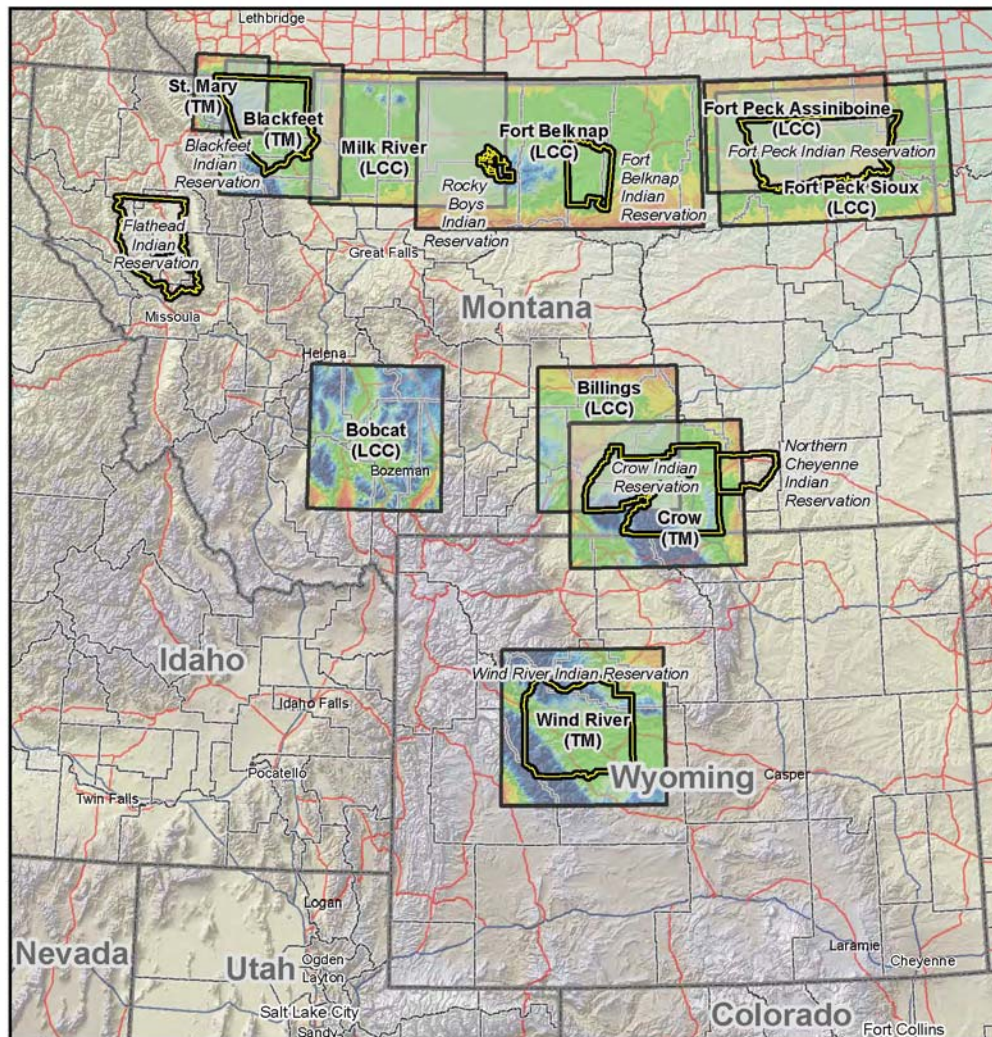


### 3.1.3 Picking a Zone to Use for a Survey/Engineering/GIS/Mapping Project


Some of RMTCRS map projection zones have zone overlap. Overlap allows users maximum choice in picking a zone to work in for their projects. For working in an overlap area, the users' goal would be to pick a zone that provides the least distortion in the project area, which often is correlated with elevation. For example, the Fort Peck Assiniboine High Zone projection scale factor is larger (higher) than the Fort Peck Sioux Low Zone projection so if you're working in that overlap area at a relative higher elevation it would be best to use the Fort Peck Assiniboine High Zone.

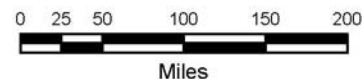
Figure 3.1.3 shows all current RMTCRS zones as boxes which are displayed in their correct locations. The size of each box considers the areas of low distortion coverage as appropriate. The boxes are not the absolute limits of the projections and there may be areas outside the boxes (and the included map set in Appendix A) where the zone coordinate system will still function well within the  $\pm 10$  to 20 ppm level.

Figure 3.1.3



**Montana and Wyoming Low Distortion Projection (LDP) Coordinate Systems**

TM = Transverse Mercator  
 LCC = Lambert Conformal Conic  
 Indian reservation



## Chapter 4 Using the RMTCRS in Software Programs

### 4.1 Adding an RMTCRS Zone Projection and Coordinate System to Software

When processing baselines and adjusting networks for projects it will be necessary to perform adjustments and input collected data from the field into projects created in certain vendor software. Input these RMTCRS zones into the appropriate 'coordinate system management/definition' module of that software. This chapter is designed to get you started, but it is recommended that you consult the 'help' documentation and tutorials of each piece of vendor software you plan to work with.

*For the purposes of entering these low distortion projection parameters into particular vendor software, normally define the datum as NAD 83 (which uses the GRS-80 reference ellipsoid) for the RMTCRS. The software may typically assume that there are no transformation parameters (zero transform) between WGS-84 and NAD 83, and that is acceptable (but not truly correct). Later, when starting an actual project you may see that project (within the software) with the local latitudes, longitudes, and heights for control points in the appropriate project datum, adjustment, and time epoch chosen.*

The screenshots shown below illustrate the upload process into various software programs. Although the screenshots are shown for the Oregon Coordinate Reference System, the same process shall be used for the RMCRS. Once the RMTCRS parameters are accepted and incorporated into vendor software, this section will be updated with RMTCRS screen shots.

#### 4.1.1 Trimble Coordinate System Manager

Trimble has created \*.csd projection files to use with field and office software. The files and an Operation and Procedure Guide can be downloaded at [www.axium.com](http://www.axium.com). Contact Amy Darlinton at [Amy.Darlinton@neciusa.com](mailto:Amy.Darlinton@neciusa.com) for an access code. The \*.csd projection files and the Operation and Procedure Guide may also be obtained by contacting Kevin McKenzie with Frontier Precision at [kmckenzie@frontierprecision.com](mailto:kmckenzie@frontierprecision.com).

#### 4.1.2 Carlson

Carlson is working to add the projections to the software drop down menus. In the meantime Jim Reinbold has created \*.csl files available upon request. The \*.csl files can also be downloaded at [www.axium.com](http://www.axium.com) along with a procedure on how to upload the files into Carlson data collectors. Contact [Amy.Darlinton@neciusa.com](mailto:Amy.Darlinton@neciusa.com) for the access code to [www.axium.com](http://www.axium.com).

### 4.1.3 Topcon Magnet Office Tools (version 2.6)

As shown below, Topcon has developed a procedure to define the RMTCRS zones in Magnet Office Tools Software. Projection input parameters for RMTCRS zones are provided in Table 3.1.1. Contact Todd Ferris at RDO Integrated Controls in Billings, Montana, (406) 794-8747 or TFerris@rdoic.com, for support.

Projections are selected and defined in the Job Configuration option of the Job Menu. Create a custom projection by clicking the Custom button.

Click on the Add button to create a new custom projection.

Name	Region	Datum	Note
Wind River	Wind River Ind...	NAD83_NO_TR...	Transverse Me...

**New Custom Projection : Projection**

**General**

Name: Wind River

Projection Type: Transverse-Mercator

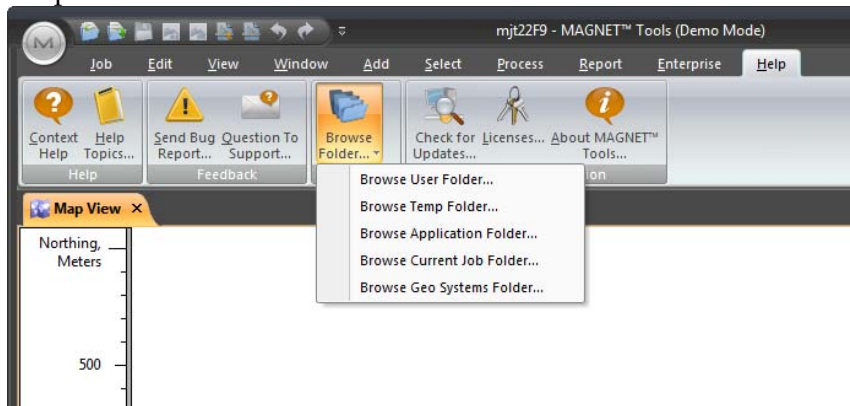
Name	Value
Central meridian	108°20'00.000000"
Scale	1.000240
Lat0	42°40'00.000000"
East0 (m)	100000.00
North0 (m)	0.00

Region: Wind River Indian Reservation

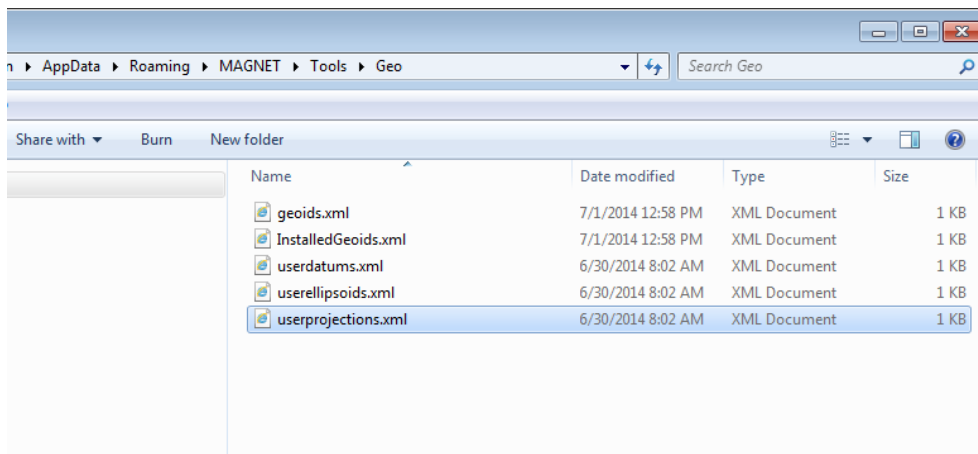
Note: Transverse Mercator Projection  
North American Datum 1983

Datum: NAD83\_NO\_TRANS

Once the Custom Projection file(s) have been created, these files can be shared with additional users. To copy the Custom Projection file(s), select Browse Folder/Browse User Folder from the Help menu.

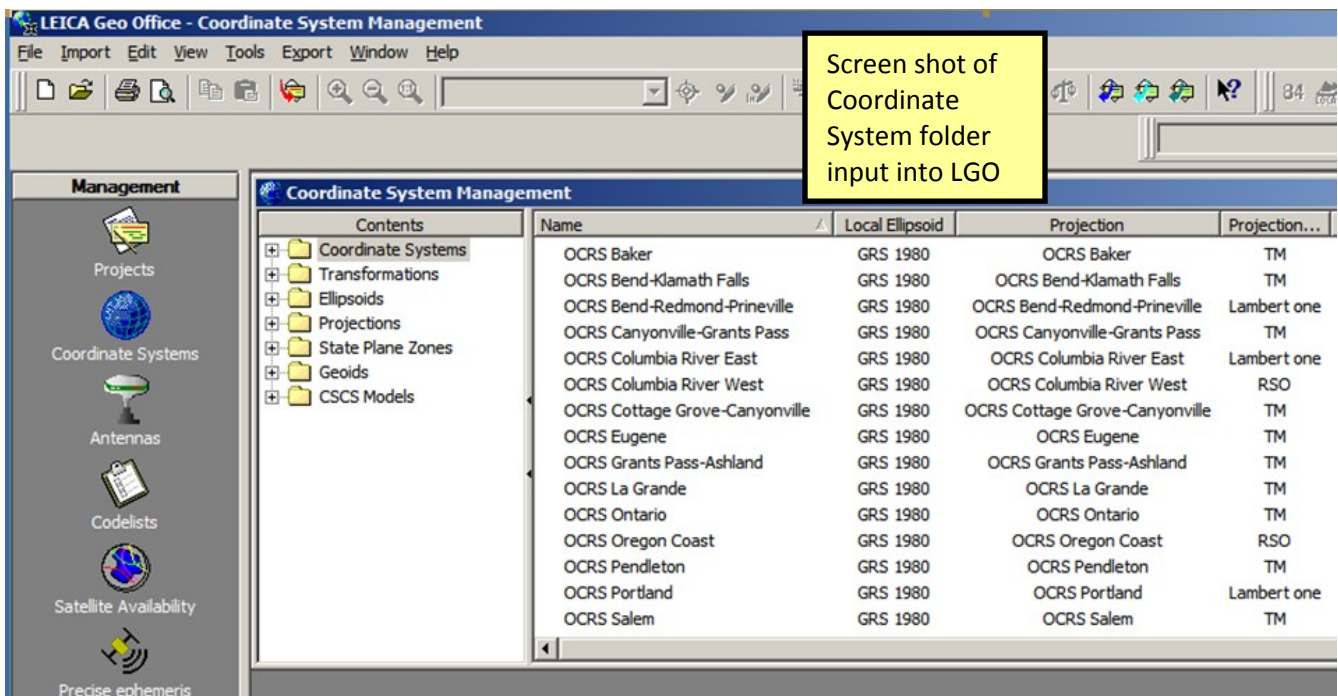
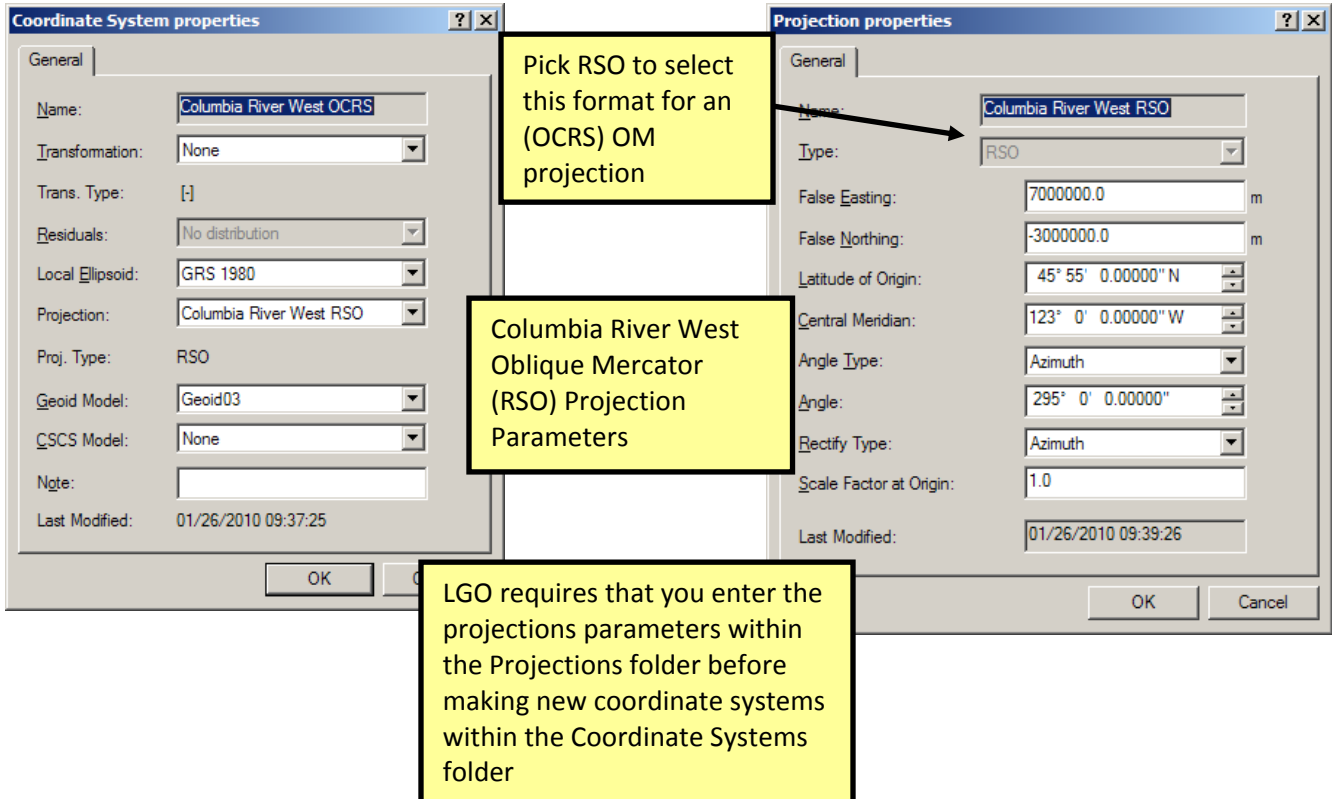


Navigate to the 'Geo' folder and find the file 'userprojections.xml'. This is the file that contains the parameters for the Custom Projection created above. You can then make a copy of this file to distribute to additional users.



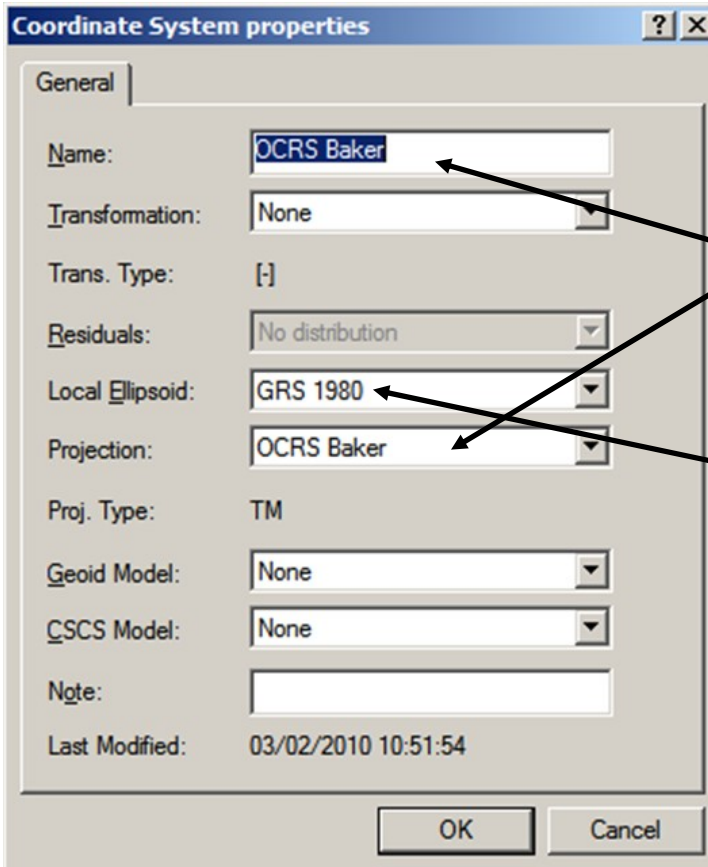
### 4.1.4 Leica Geomatics Office (LGO)

The following outlines the step-by-step procedure to add projections to the LGO. Projection input parameters for RMTCRS zones are provided in Table 3.1.1. Contact Donovan Mosser or Bryce Scala with Selby's at [dmosser@selbys.com](mailto:dmosser@selbys.com) and [bscala@selbys.com](mailto:bscala@selbys.com) for support.



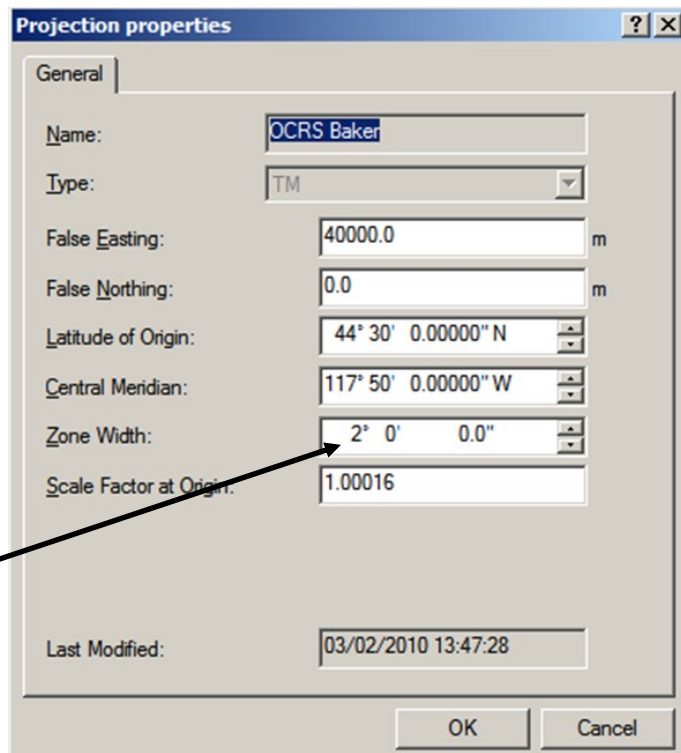


Leica (cont.)



Note that it is recommended that you name the projection with the same name as the coordinate system name to make it easy to match them up

Note: GRS-80 is the normal Local Ellipsoid choice for all OCRS zones in LGO



Note for all TM projection input into LGO it is recommended that you specify 2 degrees of coverage for the Zone Width

#### 4.1.5 ESRI ArcGIS

The projections will be available through drop down menus in both feet and meters. The Montana State Library, Geographic Information Department worked with ESRI on the projection input and assisted with the quality control process. \*.prj files may be downloaded at [www.axium.com](http://www.axium.com). Contact [Amy.Darlinton@neciusa.com](mailto:Amy.Darlinton@neciusa.com) for an access code.

#### 4.2 Checking Software Output Grid Northing's and Easting's

Table 4.2 provides the correct grid northing and easting for points in each RMTCRS zone. If you have entered the RMTCRS zone parameters into your vendor's software and successfully created coordinate systems, then, by entering the input lat/long values in the table, your project grid coordinates should match these results. The output data (northing's & easting's) in Table 4.2 are carried out to five decimal places in order to check the formulas used by each vendor. Regardless of the software, match these output values exactly (Trimble output varies in the ~last decimal place for the OM/RSO projections). If you do not match, refer back to section 4.1 and check your RMTCRS zone parameter input.

It is important for users to understand that the (local) coordinates shown in Table 4.2 are datum dependent and are shown for NAD 83(2011) or NAD(CORS). If the datum (datum realization) changes the northings and eastings will also change. Table 4.2 simply provides a coordinate check that the particular zone parameters were entered into the user's software correctly. The latitude and longitude values in the green columns represent the datum shown and the corresponding grid coordinates are shown in the output columns as metric northing and eastings.

The \*.xls files for Table 4.2 may be downloaded from [www.axium.com](http://www.axium.com). Contact [Amy.Darlinton@neciusa.com](mailto:Amy.Darlinton@neciusa.com) to obtain an access code.



Table 4.2, cont.

FMTRCS zone	N65 PID	Designation	Latitude (DMS)	Longitude (DMS)	Latitude (dec deg)	Longitude (dec deg)	Height (m)	Northing (m)	Easting (m)	Northing (ft)	Easting (ft)	Foot type	Distortion (ppm)	Combined scale factor	Convergence angle
Bobcat	DX0469	ROZEMAN SOUTHEAST	45°42'26.80572"N	111°02'55.96808"W	45.7074461667	-111.048840022222	1424.269	39,702.4454	115,664.8957	130,257.3667	379,478.0142	International	6.102	1.00001602	+07043.012"
Bobcat	DG6502	REN A	45°46'01.95996"N	111°09'22.98221"W	45.778322211111	-111.15633305022222	1345.769	47,566.2754	115,664.8957	150,265.3537	379,478.0142	International	7.862	1.00007862	+07043.58"
Bobcat	DG6503	REN B	45°47'10.41665"N	111°10'02.09200"W	45.786226941667	-111.167247777778	1339.280	48,444.0989	106,165.3005	158,937.3322	349,300.4699	International	7.605	1.00007605	+070333.20"
Bobcat	DG6504	REN C	45°46'11.51489"N	111°08'59.98310"W	45.769865241222	-111.144493950000	1349.416	46,627.2667	108,122.6826	152,976.5968	358,028.4881	International	8.351	1.00008351	+07043.4651"
Bobcat	DX0254	C160	45°37'33.49892"N	111°11'07.28000"W	45.631510811111	-111.19197072800000	1456.561	31,242.1594	104,123.0483	102,900.5229	341,627.4951	International	14.514	1.00001451	+070217.64"
Bobcat	RA0206	CWA GFS	45°51'34.11087"N	112°07'59.80333"W	45.859475241667	-112.133274380356	1376.333	56,985.8882	31,392.4569	168,895.9587	102,903.6499	International	7.664	0.999997664	+0738.1638"
Bobcat	RA0206	D 82	45°41'32.92466"N	111°25'52.40777"W	46.156419883833	-111.433327802222	1190.987	48,903.1385	98,991.4771	282,148.2156	296,003.0791	International	0.376	0.999999624	+07051.28"
Bobcat	OX0439	F145	45°41'32.92466"N	111°25'52.40777"W	45.692479072222	-111.433327802222	1373.125	38,158.2446	59,469.7461	151,191.0695	191,829.8757	International	3.610	0.999996390	+07316.23"
Bobcat	OX0439	F 575	45°29'52.89591"N	111°41'47.29300"W	45.4994157	-111.696640302278	1733.504	16,499.4157	65,094.2723	54,131.9413	213,643.8265	International	1.297	0.999998703	+07212.05"
Bobcat	OX0337	H 440	45°47'36.42858"N	111°40'29.70158"W	45.794361655556	-111.741583722222	1363.692	49,362.4005	107,807.8242	161,560.1830	202,650.1843	International	18.605	1.000018605	+072118.77"
Bobcat	OX0332	H 440	46°15'33.11822"N	112°06'41.20523"W	46.259397722222	-112.111554849056	1539.099	103,338.3315	33,569.8842	332,822.4576	110,137.4153	International	51.546	0.999948544	+07372.023"
Bobcat	RA0898	J127	46°16'58.10476"N	110°46'19.60324"W	46.282069032222	-110.772112011111	1659.130	103,758.3437	136,566.3332	340,414.5134	448,338.0884	International	69.642	0.999931358	+072042.75"
Bobcat	OX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.976218049556	-110.631253861111	1470.883	60,231.6019	349,097.1913	203,643.8381	448,989.5280	International	17.821	0.999972179	+072723.20"
Bobcat	DX0478	J 562	45°57'42.93732"N	110°37'55.21310"W	45.9762										

Table 4.2, cont.

WTCRS zone	NGS PID	Designation	Latitude (DMS)	Longitude (DMS)	Latitude (dec deg)	Longitude (dec deg)	Ellipsoid height (m)	Northing (m)	Easting (m)	Northing (ft)	Easting (ft)	Foot type	Distortion (ppm)	Combined scale factor	Convergence angle
Crow	COW0389	V 338	45°42'49.7674"N	108°47'23.9113"W	45.713824322222	-108.789891980556	1099.157	107.655795238	119.71215978	353.9084408	390.4612189	International	56.264	1.000056264	-0°24'40.31"
Crow	CV0153	Z 487	48°57'48.73545"N	107°28'06.11234"W	-07.946836458889	-107.46836458889	904.356	109.8730039	229.94136053	360.0640637	820.0312332	International	12.146	1.000012146	+0°12'24.77"
Fort Belknap	T04511	I 439	48°22'58.6092"N	108°06'52.9977"W	-08.37916679250000	-108.109592500000	674.123	135.7993571	229.3724706	445.53559485	752.5343233	International	16.847	1.000016847	+0°17'48.81"
Fort Belknap	T04744	BOUNDARY MON 370	48°59'54.14563"N	110°49'34.29236"W	-110.826195232222	-109.9444422222	1029.444	208.0170827	329.2724706	682.47017436	977.104973	International	3.477	0.999995523	+0°14'31.97"
Fort Belknap	T04745	BOUNDARY MON 380	48°59'56.24091"N	110°29'31.72469"W	-110.492145411111	-110.492145411111	845.079	206.9170827	54.2215301	682.47017436	177.89211933	International	20.799	1.000020799	+0°14'31.97"
Fort Belknap	T04746	BOUNDARY MON 388 A	48°59'56.68180"N	110°12'59.02953"W	-110.2161414571667	-110.2161414571667	875.001	206.9170827	74.3954914	678.9610163	244.0811474	International	25.573	1.000025573	+0°11'77.87"
Fort Belknap	T0486	BOUNDARY MON 398	48°59'59.15989"N	109°53'01.96511"W	-109.88387923111	-109.88387923111	850.616	206.4998814	96.7281212	677.49330491	336.59015524	International	24.756	1.000024756	-0°08'32.12"
Fort Belknap	T0487	BOUNDARY MON 407	49°00'00.69472"N	109°54'52.86822"W	-109.914693193333	-109.914693193333	803.616	206.4998814	120.8804000	676.6255747	399.59015524	International	25.918	1.000025918	+0°12'10.75"
Fort Belknap	T0488	BOUNDARY MON 418	49°00'00.47855"N	109°16'42.04923"W	-109.278347000000	-109.278347000000	872.381	205.91413184	143.9393826	675.5719107	469.3883866	International	21.402	1.000021402	-0°34'58.81"
Fort Belknap	T0489	BOUNDARY MON 427	48°59'50.37552"N	108°59'50.76828"W	-108.9954895556	-108.9954895556	914.487	205.6773295	186.0007984	674.7964474	536.14782662	International	14.715	1.000014715	-0°22'10.75"
Fort Belknap	T0490	BOUNDARY MON 437	48°59'57.90898"N	108°41'26.62569"W	-108.6907916667	-108.6907916667	919.447	205.5733495	183.00115884	674.3876294	536.14782662	International	14.715	1.000014715	-0°22'10.75"
Fort Belknap	T0491	BOUNDARY MON 447	48°59'58.39122"N	108°10'41.27401"W	-108.1711313889	-108.1711313889	882.148	205.6706141	210.98112448	674.4442720	671.7363164	International	13.905	1.000013905	-0°06'39.41"
Fort Belknap	T0492	BOUNDARY MON 456	48°59'58.86938"N	108°21'06.27407"W	-108.355186311111	-108.355186311111	821.280	205.6604678	230.9550917	674.73930676	757.72637877	International	19.339	1.000019339	+0°19'06.45"
Fort Belknap	T0493	BOUNDARY MON 465	48°59'59.26947"N	107°46'50.22688"W	-107.780618577778	-107.780618577778	864.480	206.9170827	252.661871	675.31039752	828.8916571	International	22.588	1.000022588	+0°17'48.81"
Fort Belknap	T0494	BOUNDARY MON 477	48°59'59.26947"N	107°46'29.56977"W	-107.780618577778	-107.780618577778	864.480	206.9170827	301.258798	677.6952002	988.33040519	International	33.665	1.000033665	+0°12'39.33"
Fort Belknap	T0495	BOUNDARY MON 487	49°00'00.17659"N	107°06'50.43011"W	-107.114000363889	-107.114000363889	806.470	206.53410678	301.258798	677.6952002	988.33040519	International	25.449	1.000025449	+0°27'36.97"
Fort Belknap	T0496	C 380	48°17'09.65300"N	109°06'03.81562"W	-109.101059852778	-109.101059852778	1024.026	163.3770598	155.3883975	414.8228995	508.9063868	International	33.541	0.999996649	+0°27'36.97"
Fort Belknap	SRI029	CARSON	47°41'24.22993"N	108°03'14.95011"W	-107.7281382947222	-107.7281382947222	928.410	60.02816161	233.7173594	196.33717168	766.79111599	International	83.745	1.000083745	+0°20'10.75"
Fort Belknap	T0497	CHEERY	48°52'29.69430"N	108°57'19.96652"W	-108.95540565556	-108.95540565556	1071.691	191.7967443	166.5789447	629.2544105	546.5188474	International	-26.544	0.999973456	-0°20'10.75"
Fort Belknap	T0498	F 513	48°52'49.63488"N	110°07'56.72162"W	-110.105756005556	-110.105756005556	823.975	158.8600448	88.130604	521.1943727	289.14335709	International	-8.395	0.999991605	+0°08'6.84"
Fort Belknap	T0499	HAVRE CBI 1230	48°32'47.07167"N	109°46'22.74902"W	-109.7798838889	-109.7798838889	769.633	85.9433755	106.0034943	511.0252475	677.7804623	International	-0.291	0.999999709	+0°07'12.28"
Fort Belknap	S9014	K 526	47°55'25.80011"N	108°21'16.2448"W	-108.354515222222	-108.354515222222	916.925	85.9433755	210.974950	281.9391463	691.8463216	International	26.513	1.000026513	+0°06'32.27"
Fort Belknap	T0356	L 512	48°31'25.93183"N	108°44'35.25025"W	-108.74312069444	-108.74312069444	700.675	152.6832045	167.92930252	500.9291485	597.24048564	International	10.274	1.000010274	+0°10'55.22"
Fort Belknap	T0357	LAKESIDE	48°25'34.92119"N	107°40'24.88998"W	-107.673578883333	-107.673578883333	678.403	142.1414715	261.6882886	466.44341405	856.6513405	International	14.500	1.000014500	+0°17'48.81"
Fort Belknap	SRI099	LEROY	47°53'07.15672"N	109°22'28.12878"W	-107.883321116667	-107.883321116667	1077.282	80.0154733	134.8869478	269.8805245	441.5582275	International	8.306	1.000083306	-0°39'17.81"
Fort Belknap	T0364	LONG RESET	48°57'24.31939"N	107°16'52.97861"W	-107.281382947222	-107.281382947222	928.410	201.5106048	189.2553404	661.12420297	949.0004465	International	6.287	0.999996287	+0°54'45.88"
Fort Belknap	D69749	MONTANA STATE UNI CONS ARP	48°32'27.42663"N	109°41'11.89006"W	-109.686627238889	-109.686627238889	773.920	155.2344048	112.3702085	509.2980597	368.6686631	International	-1.035	0.999998635	+0°54'45.88"
Fort Belknap	T0660	NELSON	48°30'48.80525"N	107°33'18.18204"W	-107.555050566667	-107.555050566667	693.793	151.9383984	269.8210771	498.4873635	885.2387343	International	11.294	1.000011294	+0°42'27.81"
Fort Belknap	T0661	PORTER	48°55'49.63488"N	108°02'23.54169"W	-108.0939872691667	-108.0939872691667	893.005	197.9758731	233.7193985	649.3271427	766.7978327	International	8.150	1.00008150	+0°20'40.81"
Fort Belknap	A87733	Q 93 RESET	48°14'51.62144"N	110°03'21.79456"W	-110.06054004444	-110.06054004444	798.669	124.1137658	84.4287975	403.9165543	276.9973671	International	4.480	1.00004480	+0°09'55.50"
Fort Belknap	T0359	S 512	48°34'01.15688"N	108°54'09.32222"W	-108.90434197861111	-108.90434197861111	705.988	157.8843727	167.92930252	517.9815379	551.0251931	International	10.101	1.000010101	-0°19'30.70"
Fort Belknap	T0359	SIGNAL	48°56'02.11645"N	109°54'00.72795"W	-109.900201958333	-109.900201958333	909.375	200.3305420	104.3705354	667.1700886	343.8598861	International	7.657	1.00007657	+0°58'25.66"
Fort Belknap	T0016	T 389	48°33'23.17511"N	110°25'28.11391"W	-110.424476086111	-110.424476086111	909.146	158.2533521	57.939306	519.2039310	190.0888899	International	-21.966	0.999978034	+0°12'28.85"
Fort Belknap	A87734	U 95 RESET	47°47'36.62343"N	110°32'43.95697"W	-110.94555363889	-110.94555363889	778.985	86.8201290	47.076293274	154.4511231	154.4511231	International	50.535	1.000050535	+0°13'55.31"
Fort Belknap	A87733	WHITEWATERM 2007 CONS ARP	48°43'33.86526"N	107°43'31.45670"W	-107.72504638889	-107.72504638889	815.602	175.4311531	256.9950048	575.8627596	843.1594447	International	-0.044	0.999999956	+0°34'48.50"
Fort Belknap	T0281	Y 513	48°12'13.13478"N	108°09'29.77786"W	-108.058271627778	-108.058271627778	745.850	117.1372664	232.3838369	384.9086170	763.9054677	International	16.415	1.000016415	+0°19'51.01"
Fort Belknap	T0284	Z 441	48°59'58.21670"N	107°43'31.45670"W	-107.7250463889	-107.7250463889	815.602	175.4311531	256.9950048	575.8627596	843.1594447	International	25.288	1.000025288	+0°30'18.82"
Fort Peck	T0524	B 540	48°12'26.95629"N	106°37'47.93799"W	-106.627110386111	-106.627110386111	675.517	86.6204501	116.2195730	284.1878283	381.2971832	International	16.529	1.000016529	+0°50'24.97"
Assimboie	D0389	CORPS III MILK RIVER	48°01'44.31835"N	106°19'09.96035"W	-106.319434340556	-106.319434340556	785.226	66.4797829	138.8767136	218.1095240	455.6322624	International	10.970	1.000010970	-0°36'43.69"
Fort Peck	T0332	E 91	48°31'20.48872"N	106°33'45.78979"W	-106.523259797778	-106.523259797778	811.860	121.5657445	121.9915676	398.8377445	398.8377445	International	-1.959	0.999998041	-0°47'37.93"
Fort Peck	T0022	F 92	48°50'50.97257"N	106°15'04.89233"W	-106.251358980556	-106.251358980556	922.895	157.4532726	144.8640994	516.2789783	475.2168616	International	15.654	1.000015654	-0°37'40.62"
Fort Peck	TH0751	FIRE	48°58'56.92844"N	105°04'58.83724"W	-105.083010344444	-105.083010344444	817.780	172.2803317	230.5280556	565.2241854	756.3256416	International	56.115	1.000056115	+0°18'41.41"
Fort Peck	A09828	GGW AP STA A	48°13'07.63480"N	106°37'29.35560"W	-106.624821000000	-106.624821000000	679.804	87.8744217	116.2081656	288.3013084	381.9165538	International	15.446	1.000015446	+0°50'24.97"
Fort Peck	A09826	GGW ARP	48°12'47.91642"N	106°37'08.77049"W	-106.6310116667	-106.6310116667	681.268	87.2591472	116.9241194	286.8632912	383.2814680	International	15.411	1.000015411	+0°50'05.59"
Fort Peck	T0615	GLASSGOW 2	48°12'57.73956"N	106°36'51.19599"W	-106.614221108333	-106.614221108333	685.426	87.5572805	117.1914076	287.2614190	384.4862453	International	14.661	1.000014661	-0°49'56.46"
Fort Peck	TH0424	K 542	48°06'02.97312"N	105°37'20.96571"W	-105.624904750000	-105.624904750000	593.319	74.1505511	190.8757647	243.2760864	626.2328239	International	35.204	1.000035204	-0°05'29.41"
Fort Peck	T0509	KINTYRE	48°09'40.89729"N	106°11'19.41779"W	-106.166136058333	-106.166136058333	689.177	81.1051871	148.2582812	266.0931336	488.0521035	International	16.471		

Table 4.2, cont.

Table with 17 columns: NAD83 zone, NAD83 PID, Designation, Latitude [DMS], Longitude [DMS], UTM Easting (m), UTM Northing (m), UTM Easting (ft), UTM Northing (ft), Foot type, Distortion (ppm), Combined scale factor, Convergence angle. Rows include various locations like Fort Peck, Assiniboine, and Milk River.

Table 4.2, cont.

Table with 12 columns: NGS PID, Designation, Latitude (DMS), Longitude (DMS), Longitude (dec deg), Latitude (dec deg), Ellipsoid height (m), Northing (m), Easting (m), Northing (ft), Easting (ft), Foot type, Distortion (ppm), Combined scale factor, Convergence angle.

NGS PID contained in two RMCRS's

### 4.3 Low Distortion Projects in the GIS Community

Modern GIS software incorporates on the fly projections. This allows users to simultaneously display data from differing coordinate systems in a common coordinate system on the computer screen. Low distortion projection systems can thus be easily and seamlessly incorporated for display of GIS databases. An advantage to LDPs is the fact that the historical data need not be modified. Past data can still reside in its original coordinate system and merely be re-projected in real time into the new coordinate system for use with new LDP data. Thus, as future LDPs are developed, multiple round-off error will not propagate with each time a new projection is applied. This will allow cities and counties to adopt the new LDPs while still using their original data without modification. New data can be acquired in the best LDP for the area and still be used with the historical data or other data collected by other agencies in different coordinate systems with minimal effort by the user.

Many cities and counties in the Rocky Mountain Tribal areas use GIS data to manage their resources. Thus, because LDPs generally cover the typical extents of multiple counties, a LDP will provide excellent coverage for the entire area that agency is concerned with.

GIS calculations of route distances, cut/fill volumes, etc. will be more accurate with use of LDPs because of the minimized distortion. Existing coordinate systems may be adequate for large, statewide analyses where data resolution is low (e.g. large grids cell sizes > 30m). The development of LDPs allows for new high resolution data (e.g. small grid cell sizes 0.1m to 2m) and digital terrain models (DTM) from LIDAR and other new technologies to be analyzed with minimal distortion in GIS environments when studies are performed on a localized county or city areas. Existing coordinate systems would provide a substantial amount of distortion when analyzing these DTMs. Hence, LDPs will allow for the development of more accurate GIS databases and help bridge the gap between GIS and surveying for mapping.

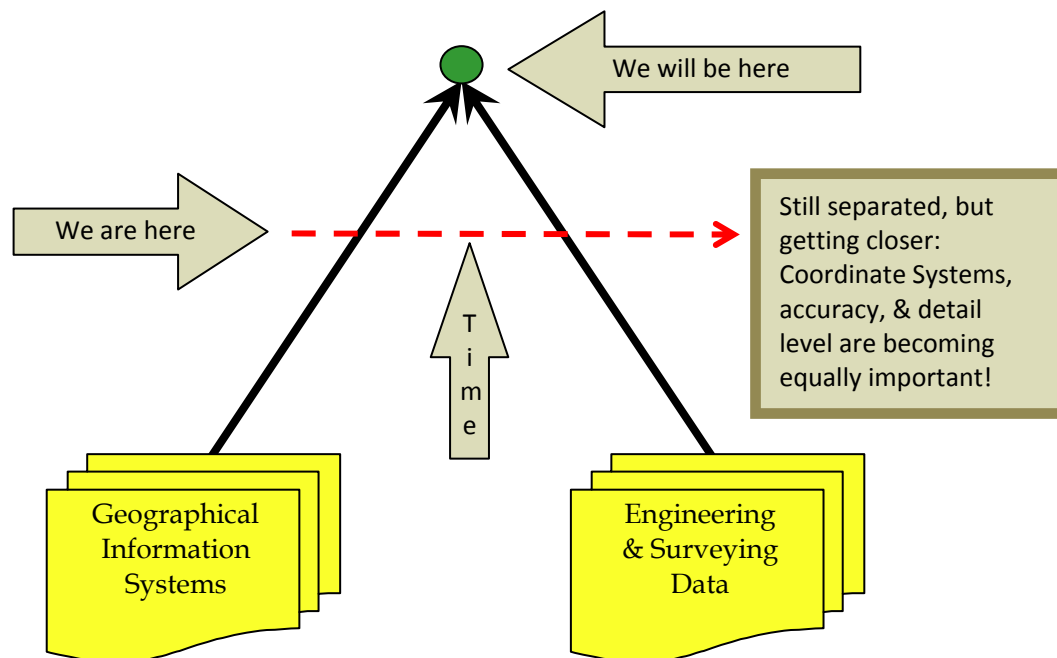


Figure 4.3, [mla,rs]



### **4.3.1 Managing GIS Data**

Geographic Information System managers administer data. Data includes spatial and attribute information that is provided from many sources. The spatial data locates features across the landscape while the attributes provide characteristics of the features. GIS managers use the same reference frameworks as surveyors to define positions in space.

Nearly all GIS operations require accurate locations of geographic features. Accurate locations allow GIS users to integrate and/or combine information from various sources. Critical to the accurate locations of features is a record of the coordinate system and associated projection parameters. GIS managers often incorporate surveyed data into geographic databases. Conversion of coordinate information into a different map projection system from which it was collected is usually necessary. Critical to this process is a well defined set of existing and desired map projection parameters.

The newly defined RMTCRS low distortion projections provide another reference system in which data will be collected. By having detailed descriptions of properties of the map projection, GIS software can re-project and transform the geographic locations of dataset elements into any appropriate coordinate system. This allows the integration of multiple GIS layers, a fundamental GIS capability.

A GIS or mapping project based on one of the new low distortion coordinate systems has significant advantages. The design of the coordinate system allows field based measurements (data collection) to be directly utilized in the GIS without translation, saving time and reducing error. The size, position and orientation of features in the system can match ground conditions, increasing confidence and reducing the need for repetitive observation.

## Chapter 5 Testing Ground vs. Grid Distances in an RMTCRS Zone

### 5.1 Testing Methods ‘Best Practices’ Adopted for RMTCRS Trial Zones

1. Field test measurements shall include measurements independent of existing Real-time GPS Networks.
2. For short (1100 m - 1300 m) and medium (3000 m – 4500 m) baseline tests, perform EDM baseline checks in each zone. Then with two GPS receivers simultaneously occupy the monuments at the ends of the baseline courses. Use NGS Calibration Baselines for short baselines as appropriate.
3. For long (30 000 m – 50 000 m) baseline tests, use paper calculation with real ground heights (CORS stations). Compare grid / ground distances in the data collector while working within the beta test projection. The curved horizontal “ground” distance may be computed by scaling the Vincenty GRS-80 ellipsoid distance to the topographic surface. Vincenty’s inverse formula will calculate the ellipsoid distance between the two points when given the latitude and longitude of each point. Then scale the resulting ellipsoid distance using the mean ellipsoid height of the end points and the geometric mean radius of curvature at the mean latitude of the endpoints.

**Step 1. Vincenty Inverse Formula<sub>(12)</sub> for ellipsoidal distance** (other variations exist):

**Use GRS-80 ellipsoid parameters:**

**[  $a = 6\,378\,137\text{ m}$ ,  $b = 6\,356\,752.314140\text{ m}$ ,  $f = 1/298.257222101$  ]**

$a$  = ellipsoid semi-major axis (= 6 378 137 m for GRS-80 ellipsoid)

$f$  = ellipsoid flattening (= 1 / 298.257222101 for GRS-80 ellipsoid)

$b = a(1 - f)$  = ellipsoid semi-minor axis

$\phi_1, \phi_2$  = geodetic latitude at end points  $p_1$  and  $p_2$  (positive north of equator)

$L$  = difference in longitude (positive east)

$\lambda$  = difference in longitude on an auxiliary sphere

$s$  = length of the geodesic (distance on ellipsoid), in the same units as  $a$

$\alpha_1$  is the initial bearing, or forward azimuth (clockwise from north)

$\alpha_2$  is the final bearing (in direction  $p_1 \rightarrow p_2$ )

$U$  = reduced latitude, where

$$U_1 = \text{atan}((1-f) \cdot \tan \phi_1)$$

$$U_2 = \text{atan}((1-f) \cdot \tan \phi_2)$$

Begin with initial approximation  $\lambda' = L$

Then iterate until change in  $\lambda'$  is negligible (e.g.  $10^{-12} \approx 0.06\text{ mm}$ ):

$$\left\{ \begin{array}{l} \sin \sigma = \sqrt{(\cos U_2 \cdot \sin \lambda)^2 + (\cos U_1 \cdot \sin U_2 - \sin U_1 \cdot \cos U_2 \cdot \cos \lambda)^2} \\ \cos \sigma = \sin U_1 \cdot \sin U_2 + \cos U_1 \cdot \cos U_2 \cdot \cos \lambda \\ \sigma = \text{atan}(\sin \sigma / \cos \sigma) \\ \sin \alpha = \cos U_1 \cdot \cos U_2 \cdot \sin \lambda / \sin \sigma \\ \cos 2\sigma_m = \cos \sigma - 2 \cdot \sin U_1 \cdot \sin U_2 / \cos^2 \alpha \\ C = (f/16) \cdot \cos^2 \alpha \cdot [4 + f \cdot (4 - 3 \cdot \cos^2 \alpha)] \\ \lambda' = L + (1 - C) \cdot f \cdot \sin \alpha \cdot \{ \sigma + C \cdot \sin \sigma \cdot [\cos 2\sigma_m + C \cdot \cos \sigma \cdot (-1 + 2 \cdot \cos^2 2\sigma_m)] \} \end{array} \right.$$

$$u^2 = \cos^2 \alpha \cdot (a^2 - b^2) / b^2$$

$$A = (1 + u^2 / 16384) \cdot \{ 4096 + u^2 \cdot [-768 + u^2 \cdot (320 - 175 \cdot u^2)] \}$$

$$B = (u^2 / 1024) \cdot \{ 256 + u^2 \cdot [-128 + u^2 \cdot (74 - 47 \cdot u^2)] \}$$

$$\Delta \sigma = B \cdot \sin \sigma \cdot \{ \cos 2\sigma_m + B/4 \cdot [\cos \sigma \cdot (-1 + 2 \cdot \cos^2 2\sigma_m) - B/6 \cdot \cos 2\sigma_m \cdot (-3 + 4 \cdot \sin^2 \sigma) \cdot (-3 + 4 \cdot \cos^2 2\sigma_m)] \}$$

$$s = b \cdot A \cdot (\sigma - \Delta \sigma)$$

$$\alpha_1 = \text{atan}((\cos U_2 \cdot \sin \lambda) / (\cos U_1 \cdot \sin U_2 - \sin U_1 \cdot \cos U_2 \cdot \cos \lambda))$$

$$\alpha_2 = \text{atan}((\cos U_1 \cdot \sin \lambda) / (-\sin U_1 \cdot \cos U_2 + \cos U_1 \cdot \sin U_2 \cdot \cos \lambda))$$

As an alternative to using the above method, the Vincenty inverse is also available in the NGS Geodetic Toolkit ([http://www.ngs.noaa.gov/TOOLS/Inv\\_Fwd/Inv\\_Fwd.html](http://www.ngs.noaa.gov/TOOLS/Inv_Fwd/Inv_Fwd.html)).

In addition, many surveying and mapping software programs can perform this calculation (although it is recommended that commercial software be checked against the NGS version).

Now scale the Vincenty ellipsoid distance using the mean ellipsoid height of the end points and the geometric mean radius of curvature at the mean latitude of the endpoints using the following formula.

**Step 2. Ground Distance** =  $((h_1 + h_2)/2) + R_G$  /  $R_G$  x [Vincenty ellipsoid distance (meters) - from step 1 above]

Where:

$h_1$  &  $h_2$  are the ellipsoid heights of the endpoints (meters)

$R_G$  is the geometric mean ellipsoid radius of curvature (GRS-80) of the endpoints (meters)

$$R_G = \frac{a\sqrt{1-e^2}}{1-e^2 \sin^2 \varphi} =$$

Where:  $a$  = semi-major axis = 6,378,137 m (exact)

$e^2$  = first eccentricity squared =  $2f - f^2$

$f$  = geometric flattening =  $1 / 298.257222101$

4. Test RTN complete software / hardware coordinate results across test projections. Latest RTCM protocol does support one standard parallel Lambert Projection. Using the RTN, test 30 to 50 km baseline lengths across zones to prove projection distortion meets predicted tolerances/ppm thresholds (pending).

## 5.2 RMTCRS Field and Office Test Methods

As part of the development of low distortion projections for the Tribal Coordinate System, field tests and calculations were employed to compare grid distances measured with GPS between two distinct points while working in a project defined by a Tribal LDP coordinate system with the direct distance measured on the ground between the same two points. If the two comparative distances were less than or equal to the projections designed threshold of, say,  $\pm 10$  ppm, then the goal was met.

Short, medium and long baselines were chosen to simulate the extreme limits of how people might use the projections. The short baselines chosen were on NGS Calibrated Baselines (CBL). For this test two baselines were set (temporary points) and the horizontal ground distance (previously checked) measured with both Trimble and CHC GPS equipment. The average of those measurements was again compared with multiple fast static GPS measurements and then processed with baseline processing software (Trimble Geomatics Office) while in the particular grid zone coordinate system. The grid vs. ground distances were then compared to see if the threshold was achieved.

For the test on long baseline lengths of  $\sim 20\,000$  m to  $\sim 80\,000$  m, one of the goals was to choose particular points beyond the edge of the planned useful area of the zone to 'break' the desired threshold and prove that it fails where it should fail (i.e., exceed the ppm design threshold). For this test, random Plate Boundary Observatory (PBO) CORS station data were used. For the grid distance baseline calculation, 24 hour RINEX files were downloaded for various PBO CORS stations, and the baselines between points were processed with baseline processing software (Trimble Geomatics Office) in the particular RMTCRS zone grid coordinate system. Since the ground distances were too long to

physically measure with an EDM, the ground distances were calculated using the Vincenty Inverse Formula (as shown in Sec. 5.1). The curved horizontal “ground” distance was computed by scaling the Vincenty GRS-80 ellipsoid distance to the topographic surface. The scale factor to do this was computed using the mean ellipsoid height of the end points and the geometric mean radius of curvature at the mean latitude of the endpoints.

**Refer to Appendix C for samples of the baseline test results.**

## **Chapter 6** The RMTCRS and Rocky Mountain Real-Time GPS Networks

Real Time GPS Networks are not currently available in Montana and Wyoming. This chapter will be updated when Real Time GPS networks are established in Montana and Wyoming.

## Chapter 7 Legislative Adoption

### 7.1 RMTCRS Legislative Adoption

The RMTCRS is substantially complete, thoroughly tested. Coordinate reference systems are very new on the national survey scene and have been generally accepted by Oregon, Minnesota, Wisconsin, and Iowa professional surveyors, engineers, GIS, cartographic, and academic professionals. Montana and Wyoming surveyors are becoming more acquainted with the use of these systems. The next step is for the Rocky Mountain Tribal Department of Transportation's (DOT) initiative is to include the Rocky Mountain Tribal Coordinate Reference System (RMTCRS) into the each participating tribe's Statutes. Legislative adoption will provide fundamental viable acceptance by engineering, surveying, and mapping professionals within the tribes as well as other Federal agencies such as the BLM, NGS and FEMA etc.

# References

## Federal and academic documents

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[http://www.mdt.mt.gov/other/survey/external/survey/manual\\_guides\\_forms/survey\\_manual/survey\\_manual\\_entire\\_manual.pdf](http://www.mdt.mt.gov/other/survey/external/survey/manual_guides_forms/survey_manual/survey_manual_entire_manual.pdf).
2. "Wyoming Department of Transportation Survey Manual"  
[https://www.dot.state.wy.us/home/engineering\\_technical\\_programs/photos\\_and\\_surveys/SurveyManual.html](https://www.dot.state.wy.us/home/engineering_technical_programs/photos_and_surveys/SurveyManual.html).
3. Federal Geographic Data Committee (1998) *Geospatial Positioning Accuracy Standards*, FGDC-STD-007.2-1998, Federal Geographic Data Committee, Reston, Virginia, USA, 128 pp.,  
<http://www.fgdc.gov/standards/projects/FGDC-standards-projects/accuracy/>, [includes Standards for Geodetic Networks (Part 2), National Standard for Spatial Data Accuracy (Part 3), and Standards for Architecture, Engineering, Construction (A/E/C) and Facility Management (Part 4)].
4. National Geodetic Survey, *User Guidelines for Single Base Real Time GNSS Positioning v3.0*, by William Henning, Lead Author
5. Snyder, J.P. (1987) *Map Projections - A Working Manual*, U.S. Geological Survey Professional Paper 1395, U.S. Government Printing Office, Washington, D.C., USA, 383 pp.
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7. "Transforming Position and Velocities between the International Terrestrial Reference Frame of 2000 and North American Datum of 1983", by Tomas Soler and Richard Snay.
8. National Imagery and Mapping Agency, 2000, *Department of Defense World Geodetic System of 1984: Its Definition and Relationships with Local Geodetic Systems (3rd Edition), Amendment 1, NIMA Technical Report 8350.2*, National Imagery and Mapping Agency (now the National Geospatial-Intelligence Agency), 175 pp., [http://earth-info.nga.mil/GandG/publications/tr8350.2/tr8350\\_2.html](http://earth-info.nga.mil/GandG/publications/tr8350.2/tr8350_2.html).
9. Vincenty, T., 1975. *Direct and inverse solutions of geodesics on the ellipsoid with application of nested equations*, *Survey Review*, Vol. 23, No. 176, pp. 88-93,  
[http://www.ngs.noaa.gov/PUBS\\_LIB/inverse.pdf](http://www.ngs.noaa.gov/PUBS_LIB/inverse.pdf).
10. National Geodetic Survey, *NOAA Manual NOS NGS 5, State Plane Coordinate System of 1983*, James E. Stem, 1989. [http://www.ngs.noaa.gov/PUBS\\_LIB/ManualNOSNGS5.pdf](http://www.ngs.noaa.gov/PUBS_LIB/ManualNOSNGS5.pdf)

## General website references

Control station datasheets: <http://www.ngs.noaa.gov/cgi-bin/datasheet.prl>

The Geodetic Tool Kit: <http://www.ngs.noaa.gov/TOOLS/>

Online Positioning User Service (OPUS): <http://www.ngs.noaa.gov/OPUS/>

Continuously Operating Reference Stations (CORS): <http://www.ngs.noaa.gov/CORS/>

The GEOID Page: <http://www.ngs.noaa.gov/GEOID/>

NGS State Geodetic Advisors: <http://www.ngs.noaa.gov/ADVISORS/AdvisorsIndex.shtml>

Geotools Page: <http://geotools.org/javadocs/org/geotools/referencing/operation/projection/ObliqueMercator.html>

POSC Specifications – Hotline Oblique Mercator: [http://posc.org/Epicentre.2/DataModel/ExamplesofUsage/eu\\_cs34i.html](http://posc.org/Epicentre.2/DataModel/ExamplesofUsage/eu_cs34i.html)

Radius at a given geodetic latitude: [https://visualization.hpc.mil/wiki/Radius\\_of\\_the\\_Earth](https://visualization.hpc.mil/wiki/Radius_of_the_Earth)

Vincenty Formula: <http://www.movable-type.co.uk/scripts/latlong-vincenty.html>

Helmert Transformations: <http://earth-info.nga.mil/GandG/coordsys/datums/helmert.html>

Ordnance Survey:

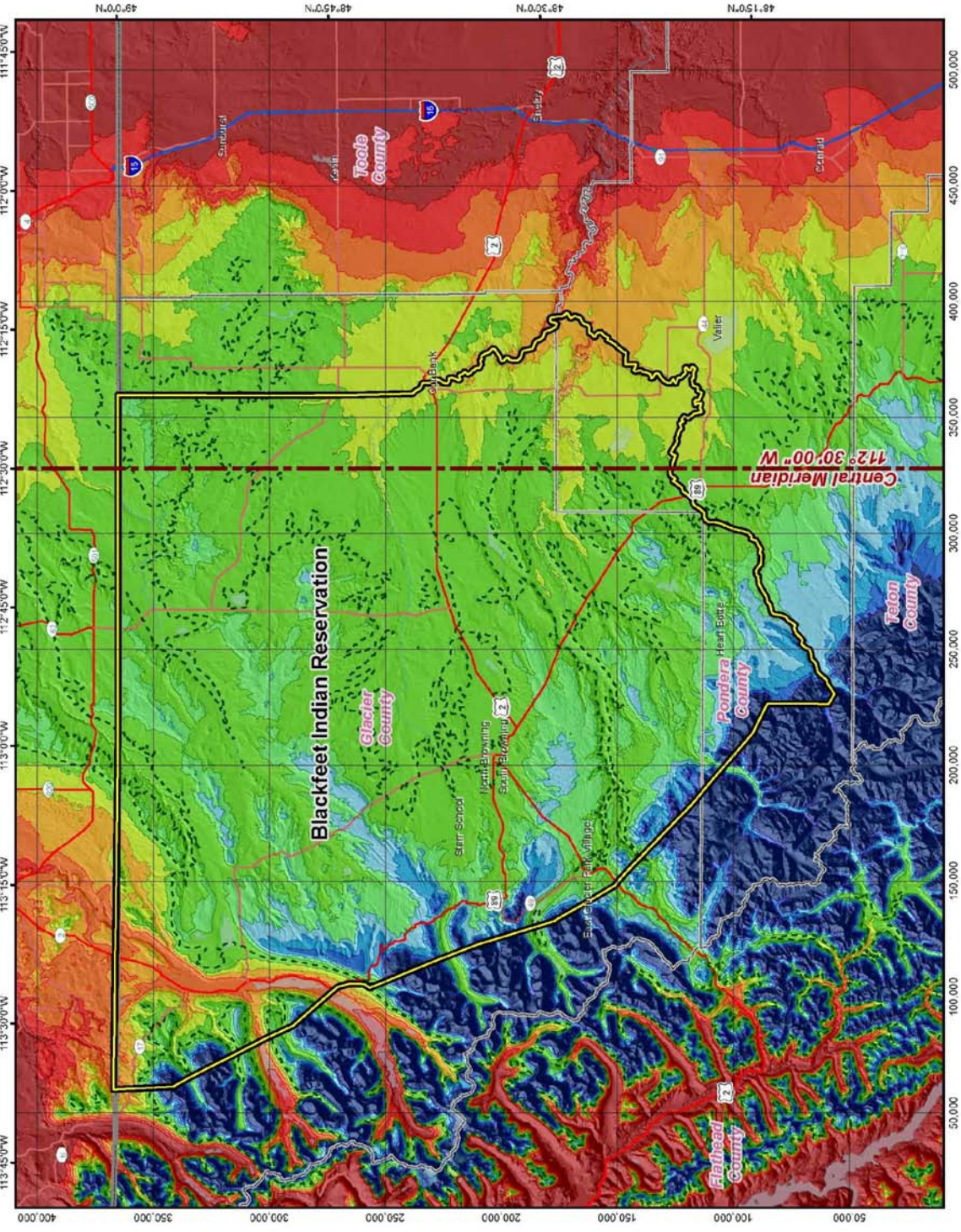
<http://www.ordnancesurvey.co.uk/oswebsite/gps/information/coordinatesystemsinfo/guidecontents/guide6.html>

Datum transformations: [http://www.niirs10.com/support/ct\\_geocue/geocue\\_ct\\_3.pdf](http://www.niirs10.com/support/ct_geocue/geocue_ct_3.pdf)



# **Appendix A**

## **RMTCRS Zone Maps**



### Blackfoot Coordinate System

Transverse Mercator Projection  
 North American Datum of 1983  
 Latitude of grid origin: 48°00'00" N  
 Central meridian: 112°30'00" W  
 False northing: 0.000 m  
 False easting: 100,000.000 m  
 Central meridian scale: 1 000 190 (exact)



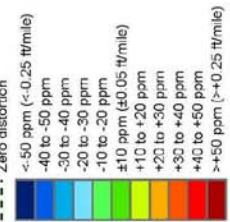
Projected map grid shown in units of international feet

Scale: 1:500,000

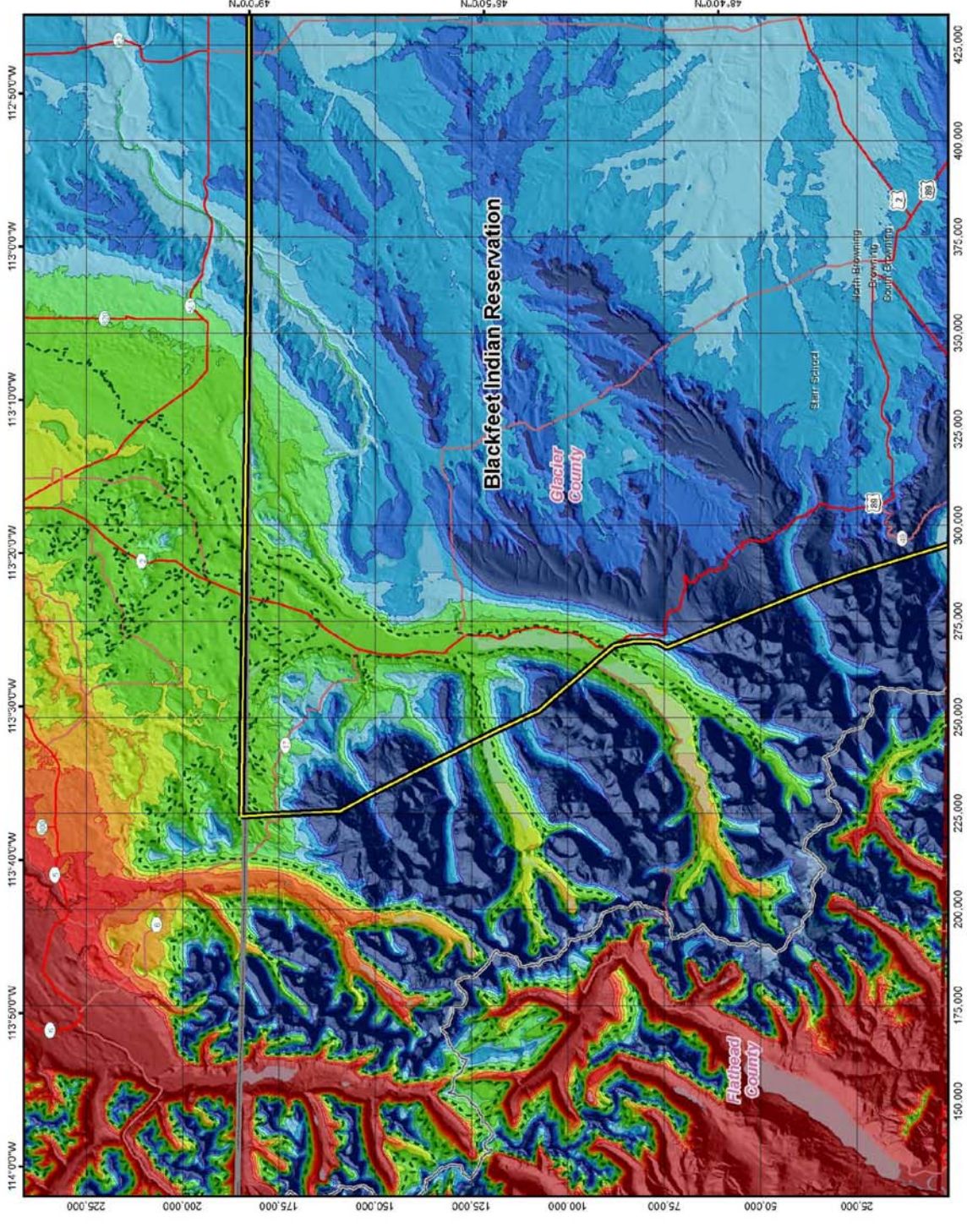
(When printed on 11" x 17" sheet)



### Linear distortion (parts per million)

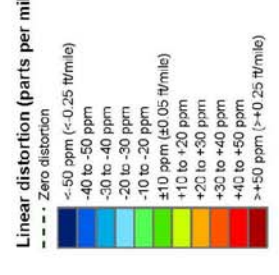
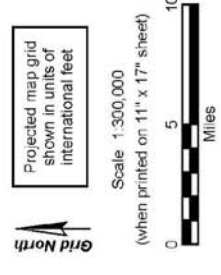


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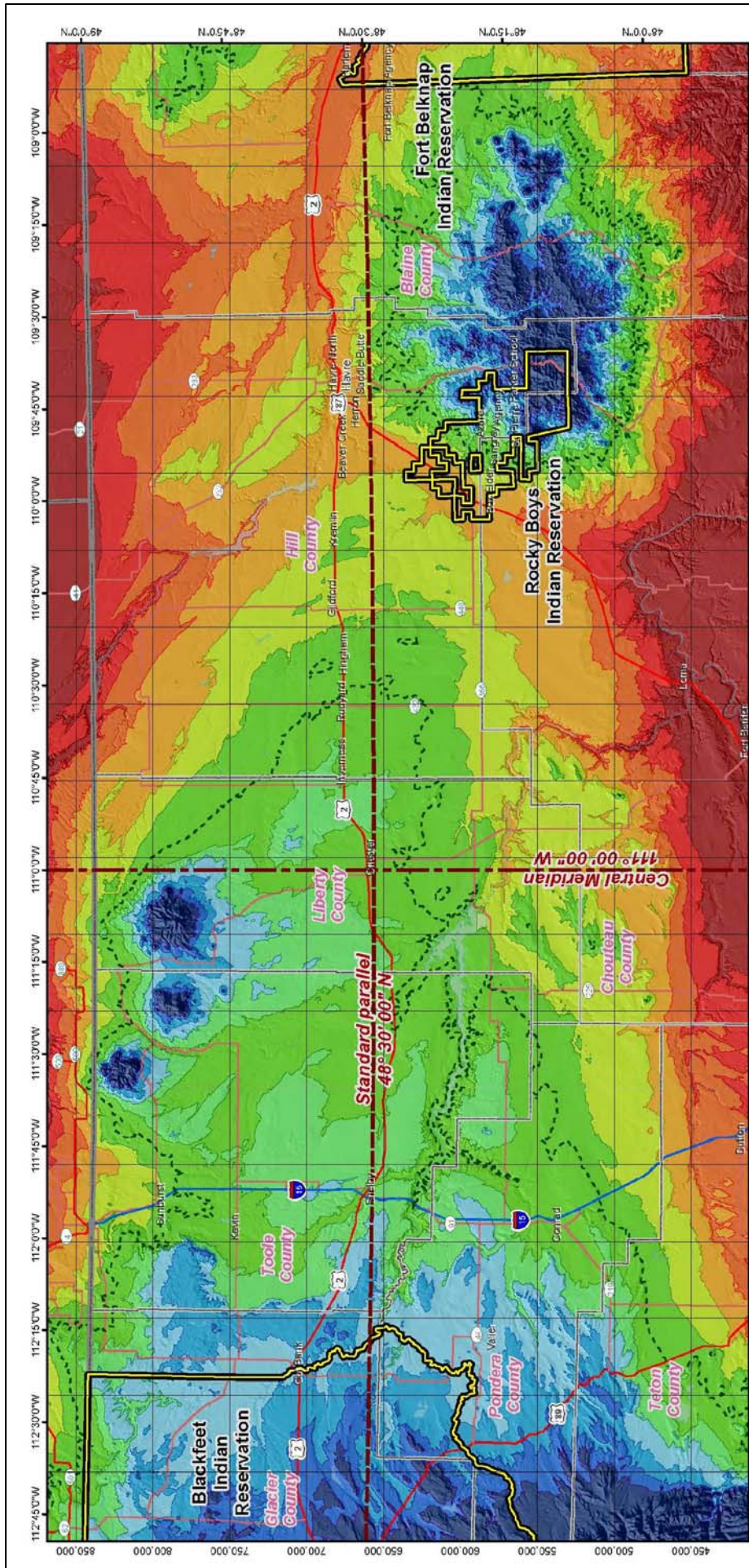
**St. Mary  
Coordinate System**

Transverse Mercator Projection  
 North American Datum of 1983  
 Latitude of grid origin: 48°30'00" N  
 Central meridian: 112°30'00" W  
 False northing: 0.000 m  
 False easting: 150 000.000 m  
 Central meridian scale: 1 000 160 (exact)

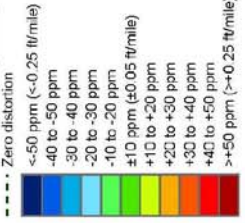


Designed and prepared by  
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 mid@geodeticanalysis.com

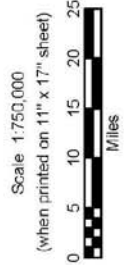
**Geodetic ANALYSIS**  
 CONSULTING, INC.



Linear distortion (parts per million)



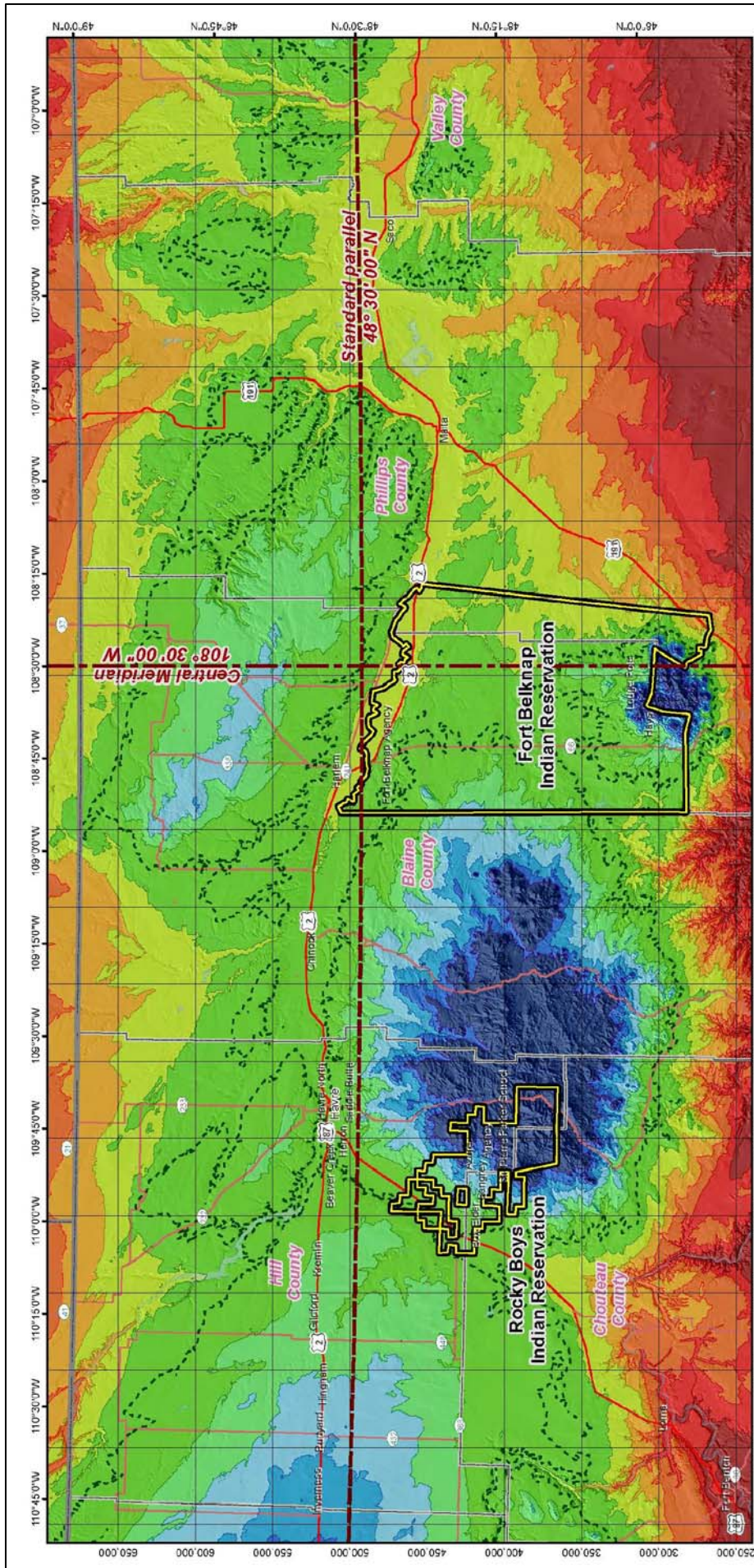
Projected map grid shown in units of International feet



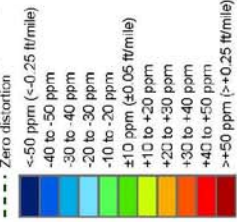
**Milk River Coordinate System**  
 Lambert Conformal Conic Projection  
 (single parallel)  
 North American Datum of 1983  
 Standard parallel & grid origin: 48° 30' 00" N  
 Central meridian: 111° 00' 00" W  
 False northing: 200,000,000 m  
 False easting: 150,000,000 m  
 Standard parallel scale: 1:1000,145 (exact)



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 mlid@geomatics.com  
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 PAKISTAN ENGINEERING & CONSULTING, INC.



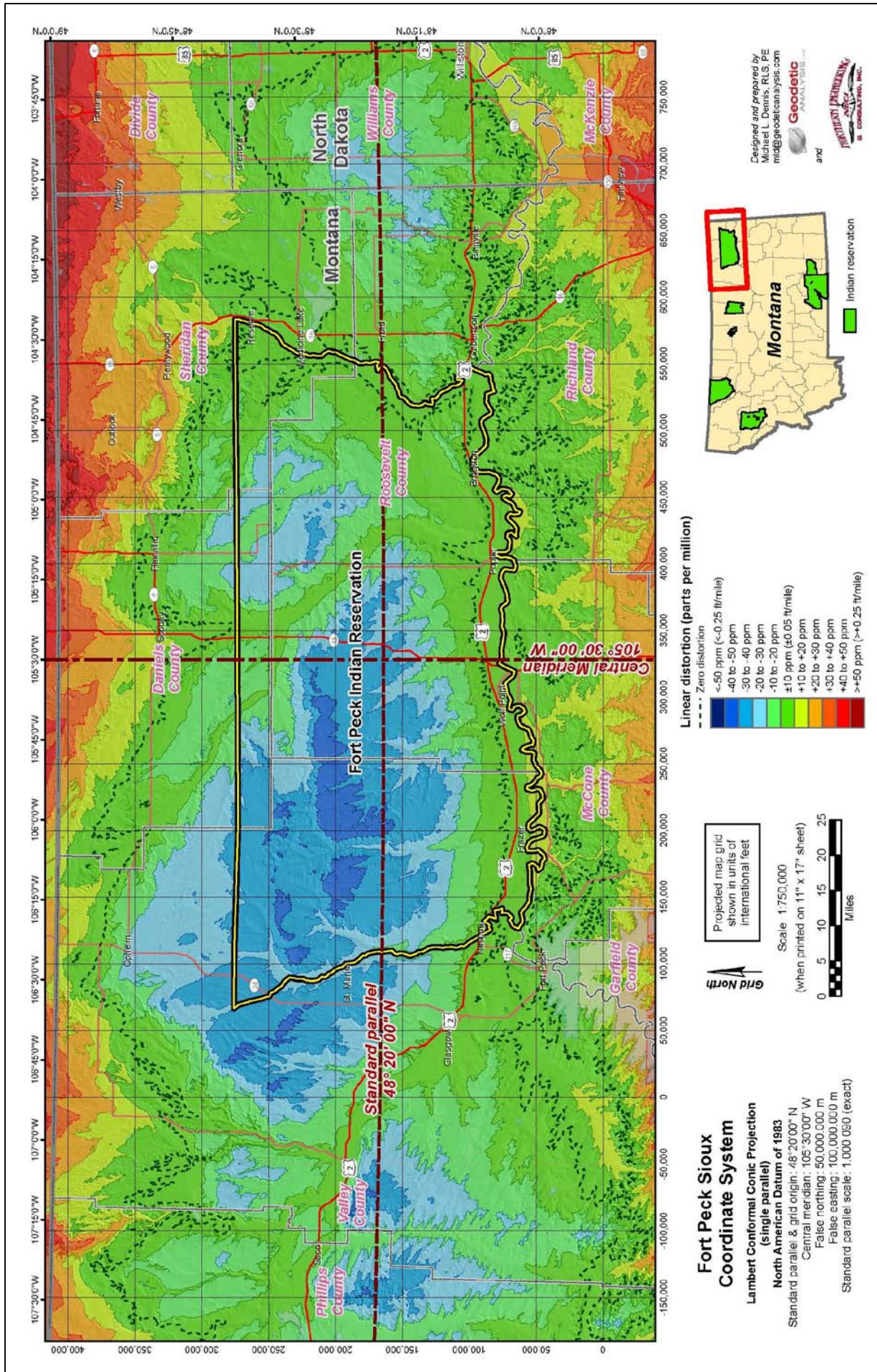
Linear distortion (parts per million)



**Fort Belknap  
Coordinate System**  
Lambert Conformal Conic Projection  
(single parallel)  
North American Datum of 1983  
Standard parallel & grid origin: 48° 30' 00" N  
Central meridian: 108° 30' 00" W  
False northing: 150,000,000 m  
False easting: 200,000,000 m  
Standard parallel scale: 1,000,120 (exact)

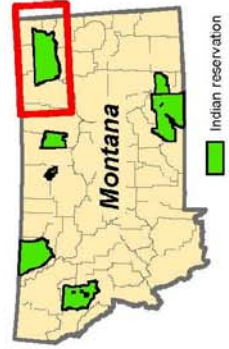
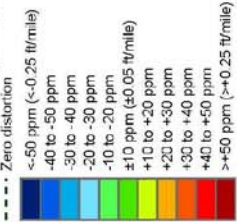
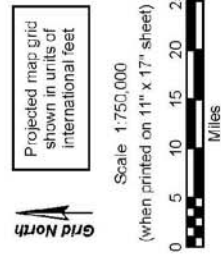


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**PROFESSIONAL ENGINEERING  
& CONSULTING, INC.**

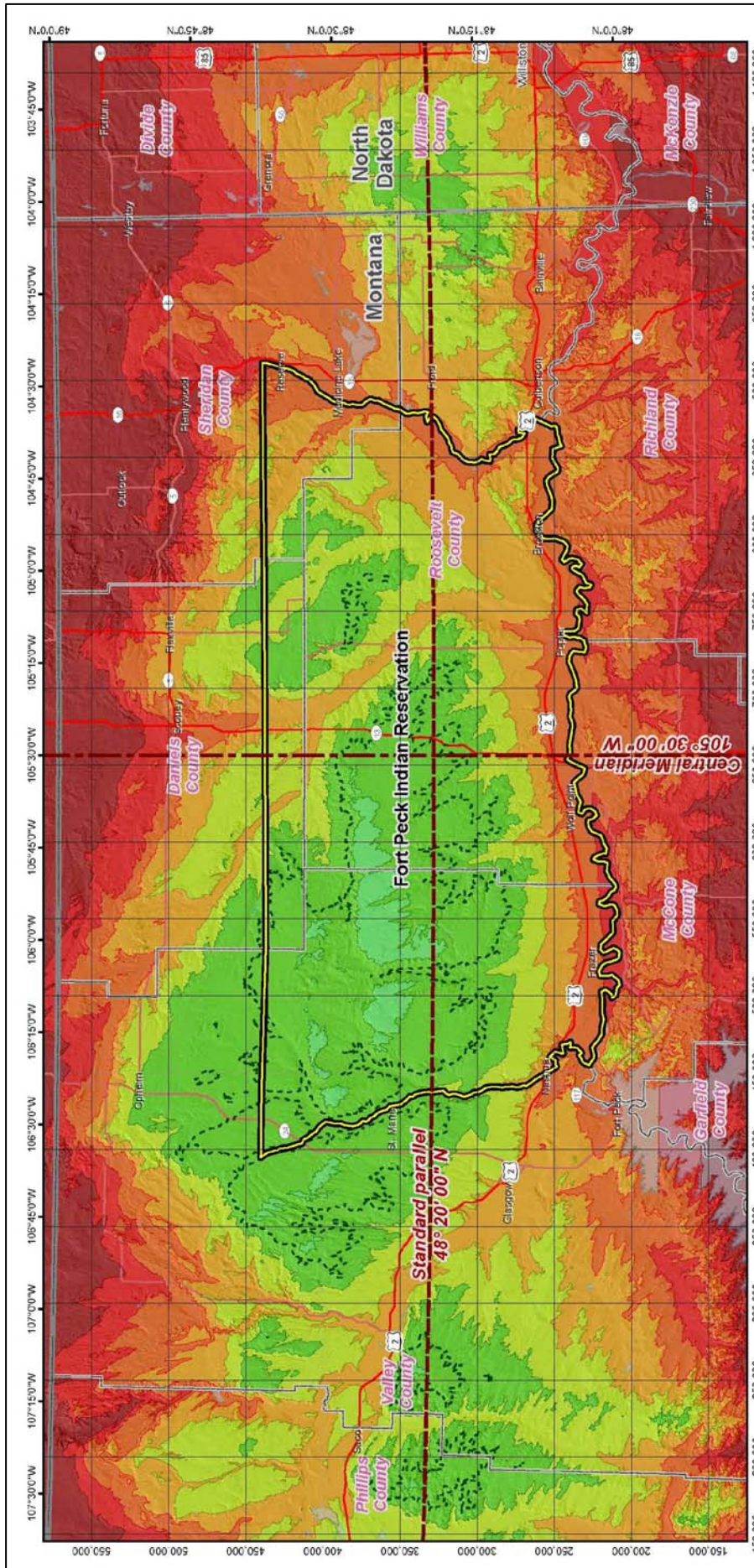


### Fort Peck Sioux Coordinate System

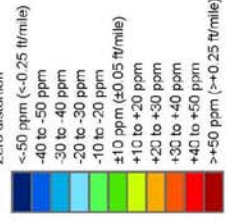
Lambert Conformal Conic Projection  
 (single parallel)  
 North American Datum of 1983  
 Standard parallel & grid origin: 48°20'00" N  
 Central meridian: 105°30'00" W  
 False northing: 50,000,000 m  
 False easting: 100,000,000 m  
 Standard parallel scale: 1:1,000,050 (exact)



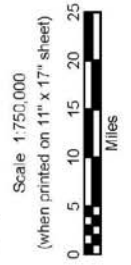
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 and  
 American Photocopying  
 CORPORATION, INC.



Linear distortion (parts per million)



Projected map grid shown in units of international feet



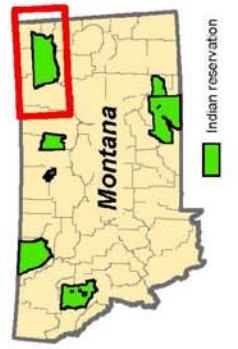
**Fort Peck Assiniboine Coordinate System**

**Lambert Conformal Conic Projection (single parallel)**  
**North American Datum of 1983**  
 Standard parallel & grid origin: 48° 20' 00" N  
 Central meridian: 105° 30' 00" W  
 False northing: 100,000,000 m  
 False easting: 200,000,000 m  
 Standard parallel scale: 1:000 012 (exact)



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 Helena, MT 59601



# Crow Coordinate System

Transverse Mercator Projection  
North American Datum of 1983  
Latitude of grid origin: 44° 45' 00" N  
Central meridian: 107° 45' 00" W  
False northing: 0 000 000 m  
False easting: 200 000 000 m  
Central meridian scale: 1 000 148 (exact)



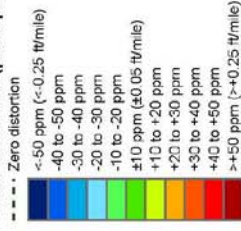
Projected map grid  
shown in units of  
international feet

Scale 1:600,000

(when printed on 11" x 17" sheet)

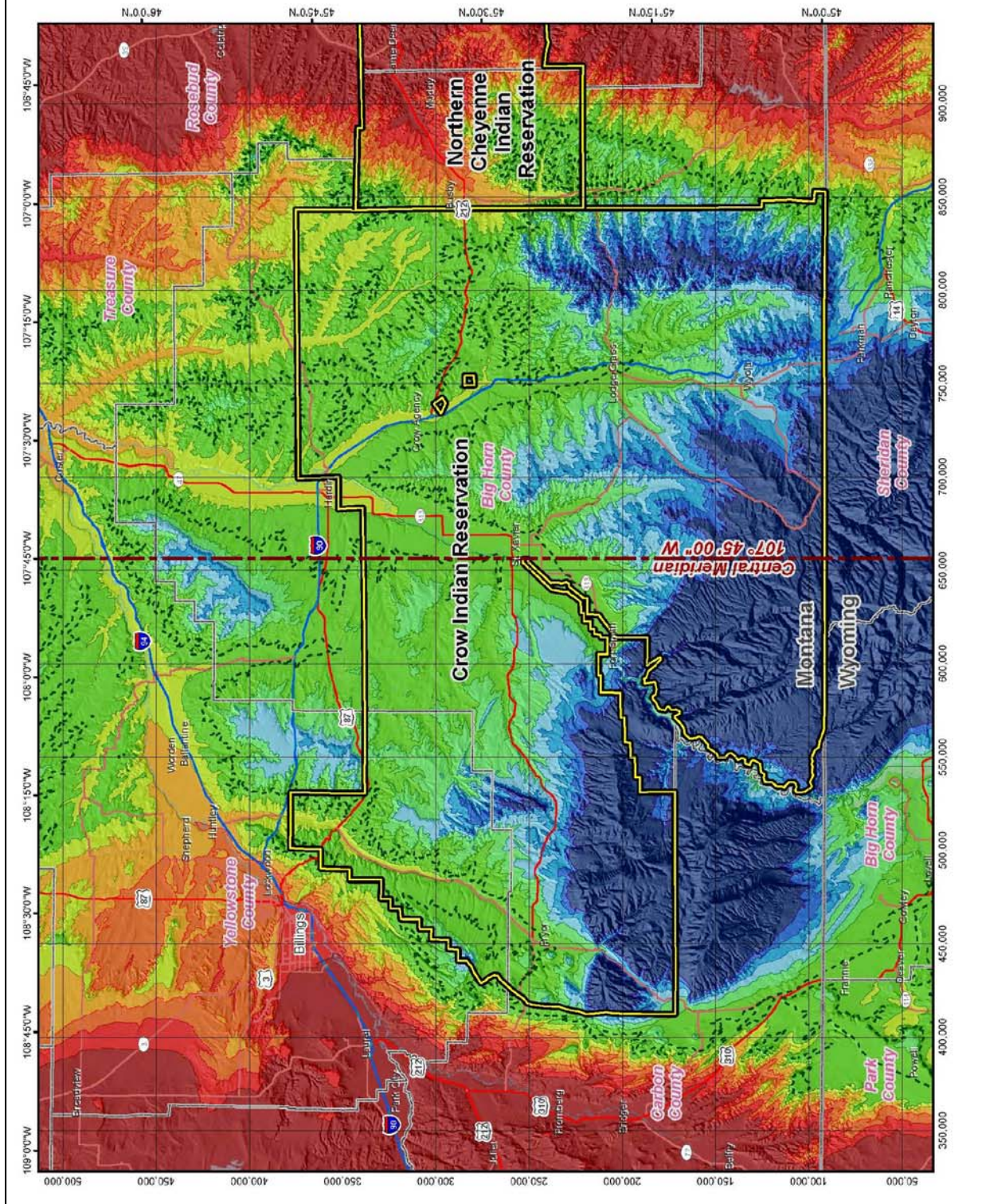


## Linear distortion (parts per million)



Indian reservation

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**Geodetic**  
ANALYSIS





# Wind River Coordinate System

Transverse Mercator Projection  
North American Datum of 1983  
Latitude of grid origin: 42° 40' 00" N  
Central meridian: 108° 20' 00" W  
False northing: 0,000 m  
False easting: 100,000,000 m  
Central meridian scale: 1,000,240 (exact)



Projected map grid  
shown in units of  
US survey feet

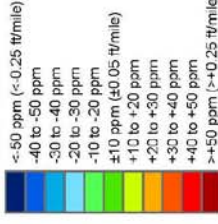
Scale 1:500,000

(when printed on 11" x 17" sheet)

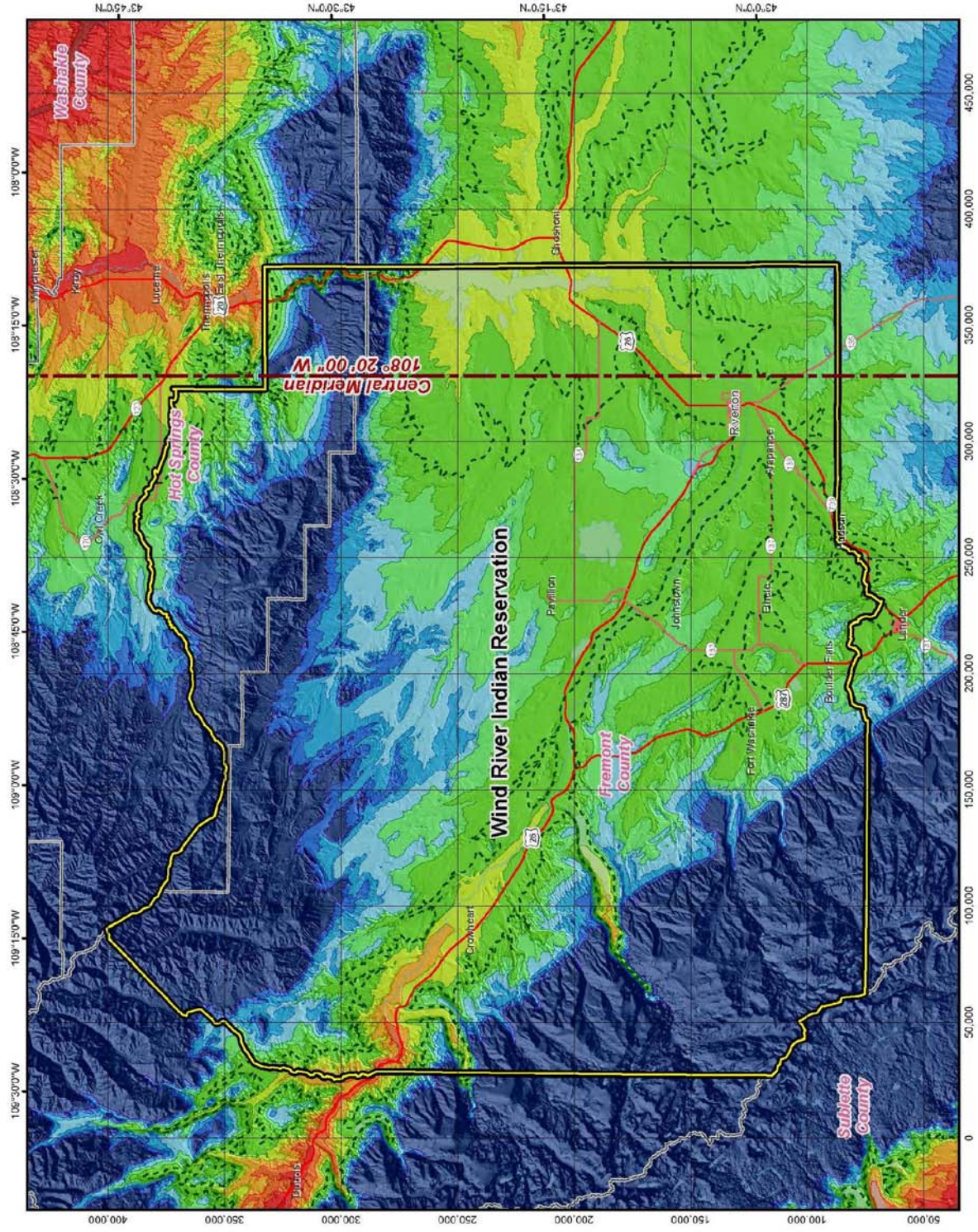


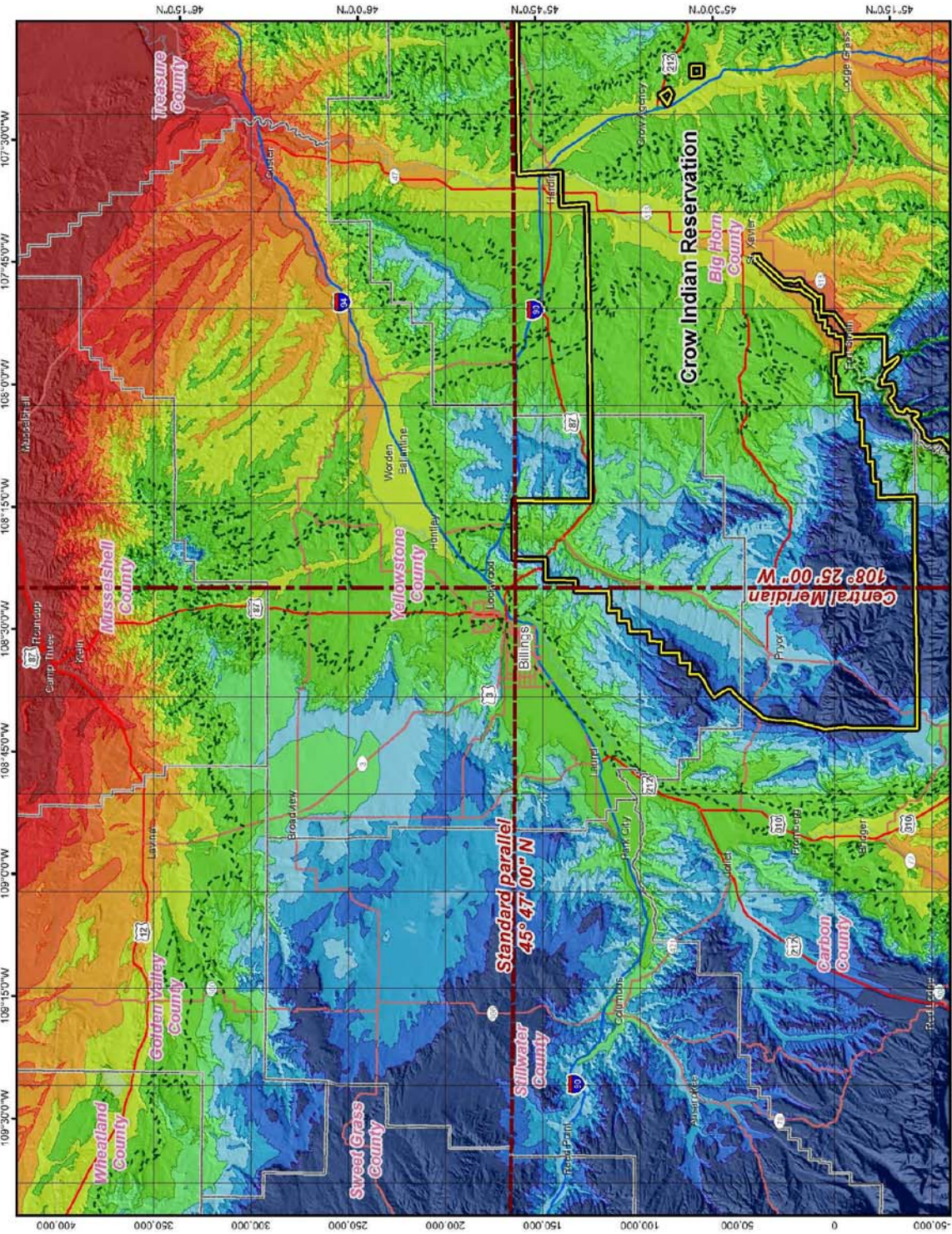
## Linear distortion (parts per million)

--- Zero distortion



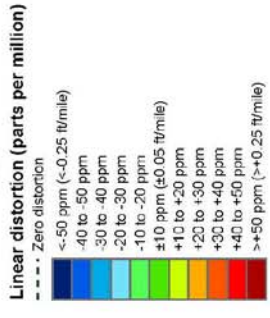
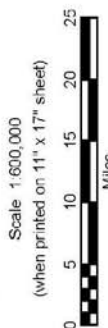
Designed and prepared by  
Michael L. Dennis, R.L.S., P.E.  
mid@geodeticanalysis.com  
**Geodetic ANALYSIS, LLC**  
A CONSULTING, INC.





# Billings Coordinate System

Lambert Conformal Conic Projection  
(single parallel)  
North American Datum of 1983  
Standard parallel & grid origin: 45°47'00" N  
Central meridian: 108°25'00" W  
False northing: 50,000,000 m  
False easting: 200,000,000 m  
Standard parallel scale: 1:000 1515 (exact)



Designed by  
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STEWART

Map prepared by  
Michael L. Dennis, RLS, PE  
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Geodetic  
ANALYSIS  
and  
PROJECT PARTNERS  
CONSULTING, INC.

# Bobcat Coordinate System

Lambert Conformal Conic Projection  
(single parallel)

North American Datum of 1983

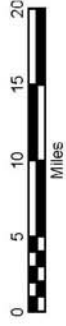
Standard parallel & grid origin: 46°15'00" N  
 Central meridian: 111°15'00" W  
 False northing: 100,000,000 m  
 False easting: 100,000,000 m  
 Standard parallel scale: 1:000 185 (exact)



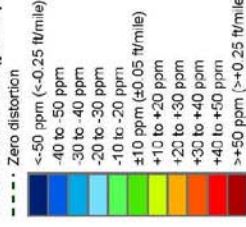
Projected map grid  
shown in units of  
international feet

Scale 1 500,000

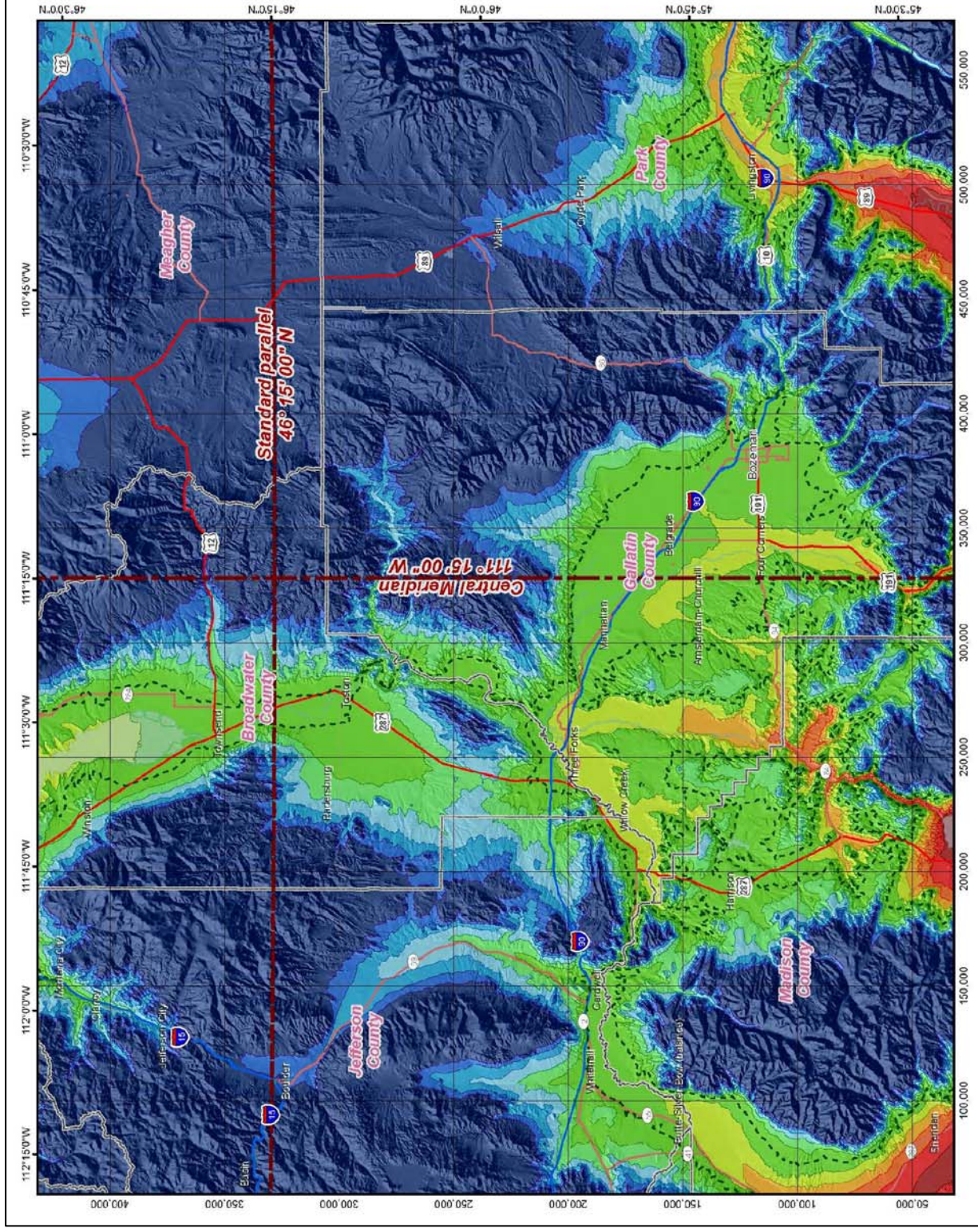
(when printed on 11" x 17" sheet)



## Linear distortion (parts per million)



Designed and prepared by  
 Rich Jensen, PLS  
 rjensen@sandersonstewart.com



# **Appendix B**

## **RMTCRS Distortion Overview Maps**

# Distortion Overview Montana and Wyoming Low Distortion Projection (LDP) Coordinate Systems

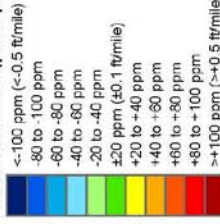
Distortion displayed for the following zones (extended coverage used to show performance beyond design area):

- Billings (LCC)
- Fort Peck Sioux (LCC)
- St. Mary (TM)

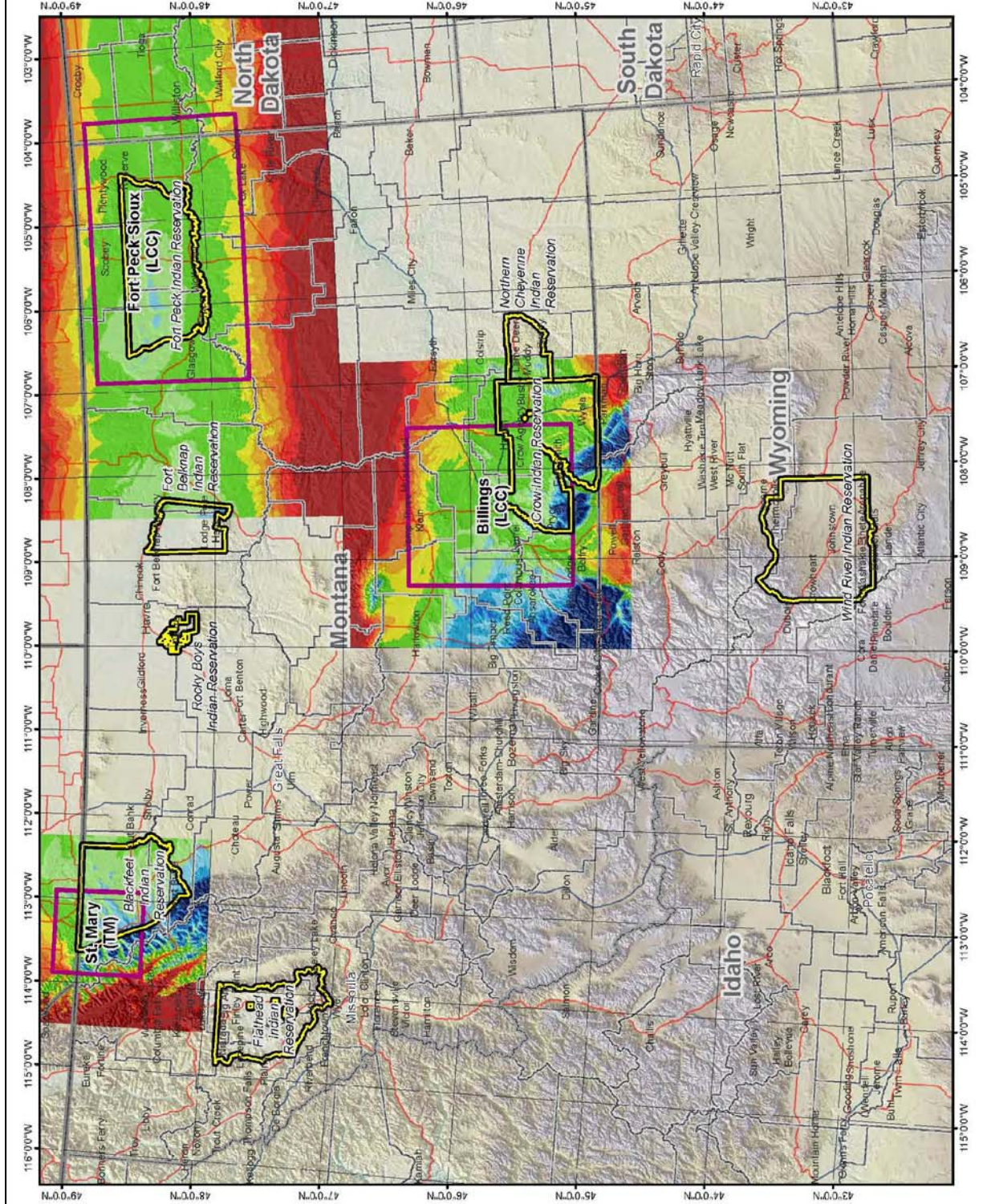
TM = Transverse Mercator projection  
LCC = Lambert Conformal Conic projection



### Linear distortion (parts per million)



Designed and prepared by  
Michael L. Dennis, RLS, PE  
ml@dgsolutions.com  
**Geodetic SOLUTIONS, INC.**



# Distortion Overview

## Montana and Wyoming Low Distortion Projection (LDP) Coordinate Systems

Distortion displayed for the following zones (extended coverage used to show performance beyond design area):

- Blackfeet (TM)
- Crow (TM)
- Fort Peck Assiniboine (LCC)

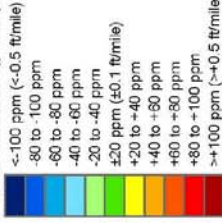
TM = Transverse Mercator projection  
LCC = Lambert Conformal Conic projection

Scale 1:3,250,000

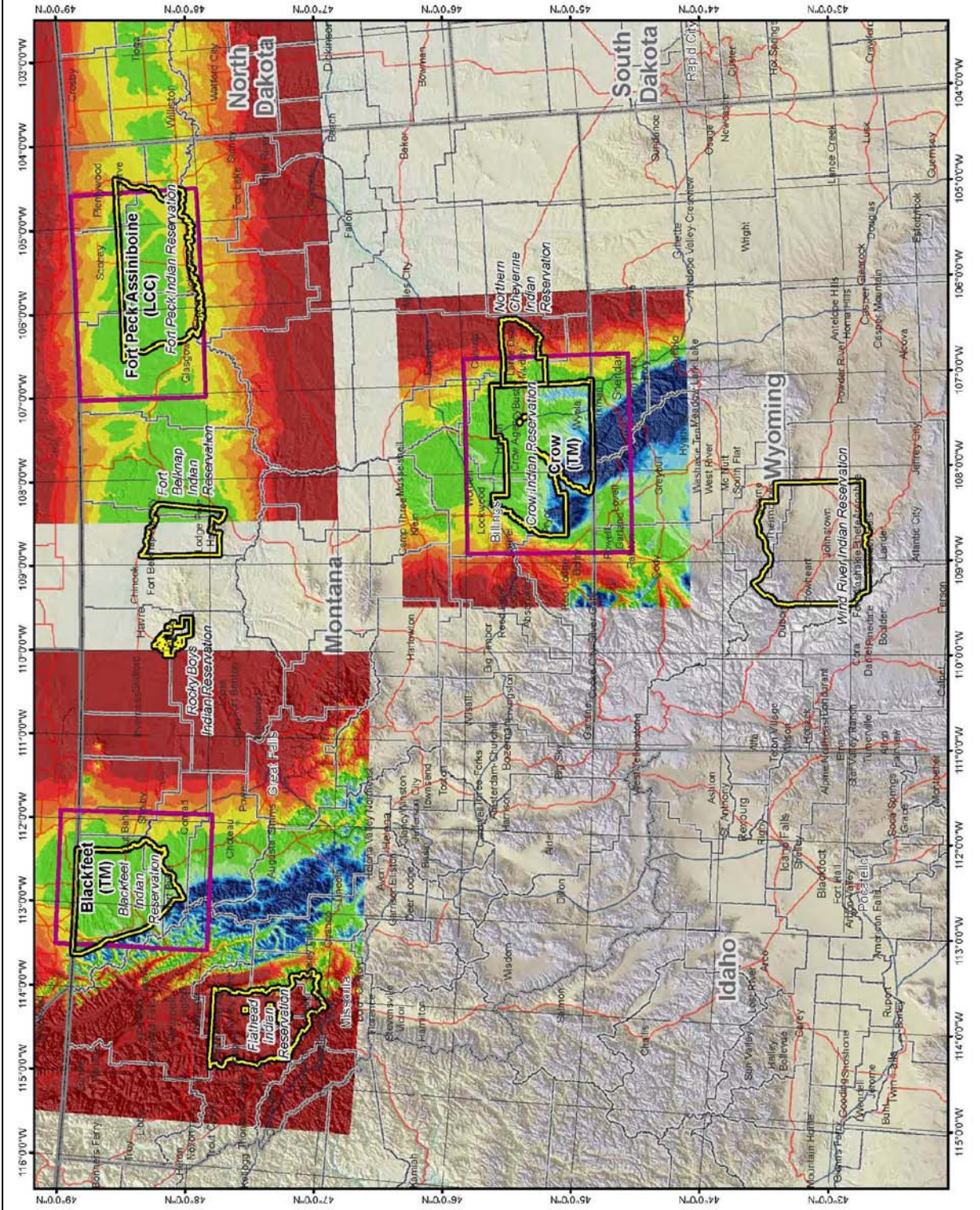
(When printed on 11" x 17" sheet)



### Linear distortion (parts per million)



Designed and prepared by  
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mid@geodeticanalysis.com  
**Geodetic ANALYSIS**  
CONSULTING, INC.

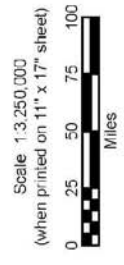


# Distortion Overview Montana and Wyoming Low Distortion Projection (LDP) Coordinate Systems

Distortion displayed for the following zones (extended coverage used to show performance beyond design area):

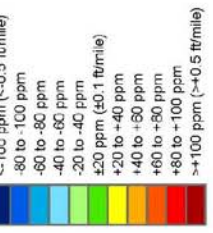
- Bobcat (LCC)
- Milk River (LCC)

TM = Transverse Mercator projection  
LCC = Lambert Conformal Conic projection



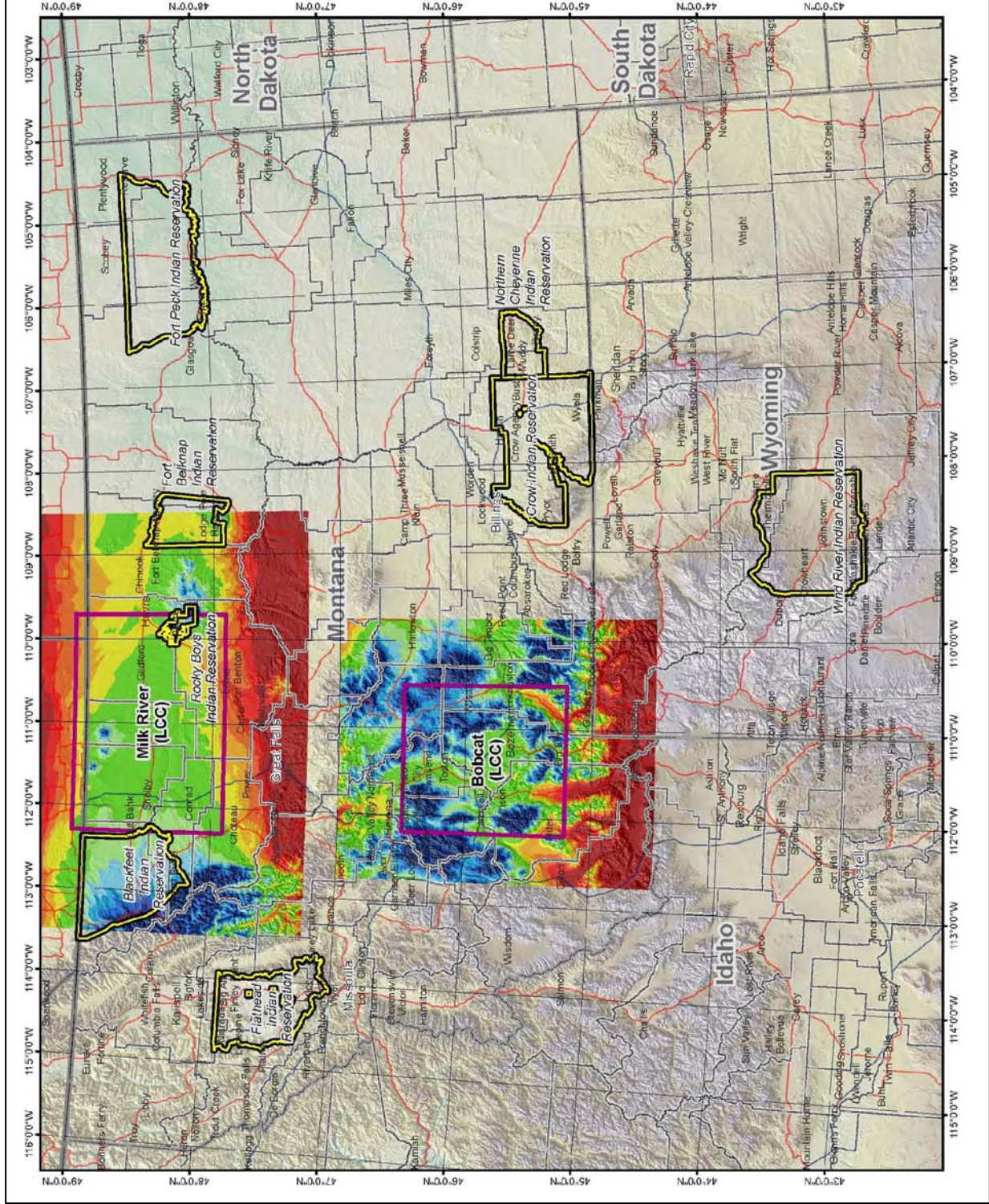
- Indian reservation
- LDP design area

### Linear distortion (parts per million)



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**Geodetic**  
ANALYSIS, INC.



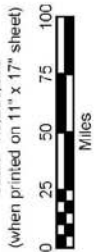
# Distortion Overview Montana and Wyoming Low Distortion Projection (LDP) Coordinate Systems

Distortion displayed for the following zones (extended coverage used to show performance beyond design area):

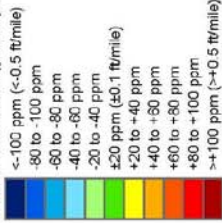
- Fort Belknap (LCC)
- Wind River (TM)

TM = Transverse Mercator projection  
LCC = Lambert Conformal Conic projection

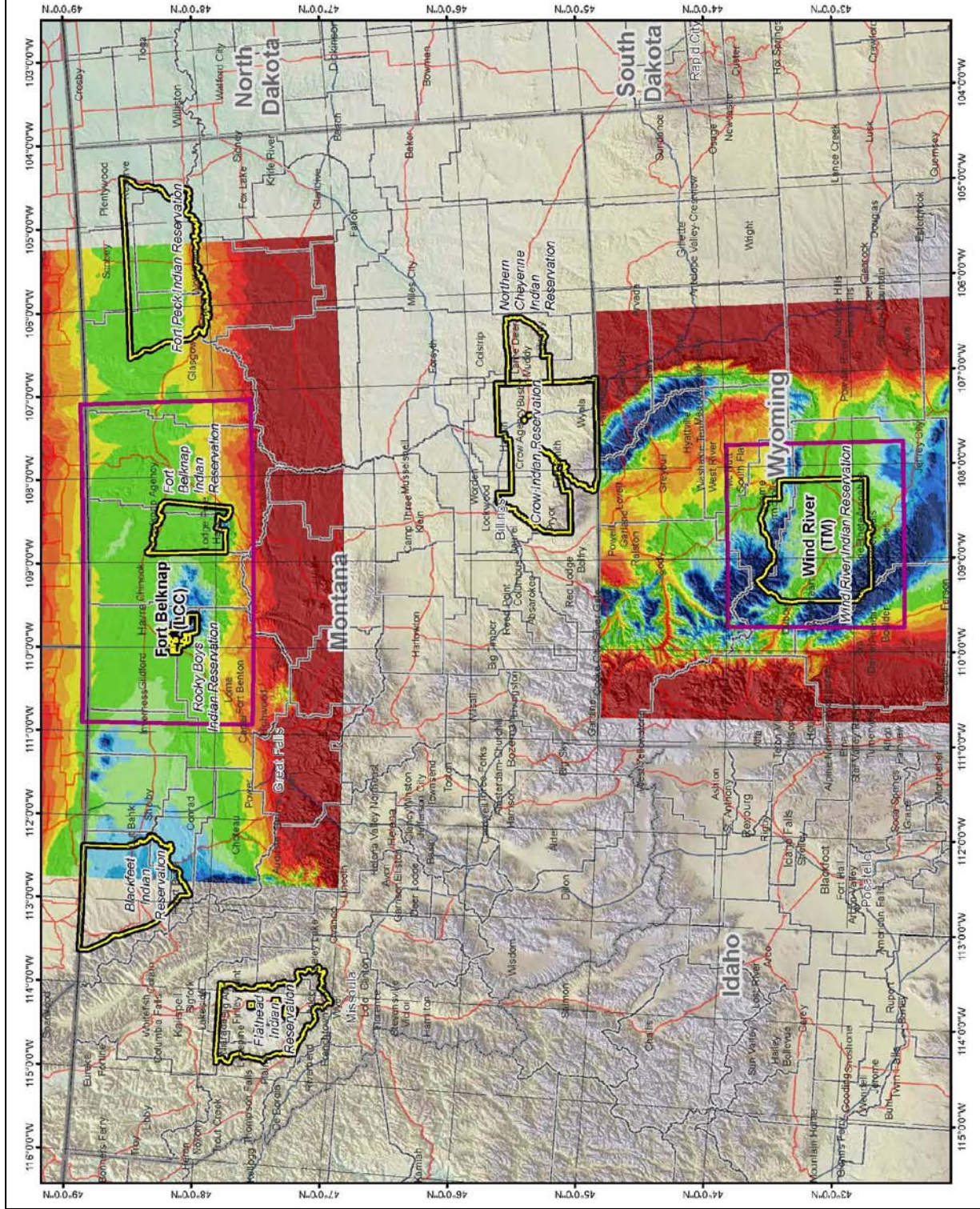
Scale 1:3,250,000



## Linear distortion (parts per million)



Designed and prepared by  
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**Geodetic**  
ANALYSIS, INC.





# **Appendix C**

## **RMTCRS Trial – Field Testing Results**

NGS GPS Station Distances																		
Blackfeet & St.	NGS Dist. To R424			NGS Dist. To Blackfeet			NGS Dist. To Sherburne 2			Observed Dist. To R424			Observed Dist. To Blackfeet			Observed Dist. To Sherburne 2		
	Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing	
Mary LDP	0.00	-	-	62940.05	N 66° 08' 07.3077" E	-	151485.66	N 24° 42' 31.2835" W	-	0.00	-	-	15041.58	N 56° 42' 47" W	-	-	-	-
R424	0.00	-	-	62940.05	N 66° 08' 07.3077" E	-	151485.66	N 24° 42' 31.2835" W	-	0.00	-	-	15041.58	N 56° 42' 47" W	-	-	-	-
Blackfeet	77500.02	S 64° 41' 47.2197" W	-	14665.23	S 58° 30' 34.4540" W	-	167719.10	N 51° 12' 19.9654" W	-	77918.13	S 62° 35' 19" W	-	0.00	-	-	26593.55	N 71° 39' 17" E	-
Sherburne 1	62940.05	S 66° 08' 07.3077" W	-	0.00	-	-	163458.09	N 46° 03' 46.2484" W	-	62939.20	S 66° 08' 07" W	-	0.00	-	-	-	-	-
Sherburne 2	157003.84	S 33° 03' 21.8294" E	-	178130.26	S 54° 38' 07.4055" E	-	265303.97	N 73° 50' 37.6931" E	-	0.00	-	-	-	-	-	-	-	-
	151485.66	S 24° 42' 31.2835" E	-	163458.09	S 46° 03' 46.2484" E	-	0.00	-	-	-	-	-	-	-	-	-	-	-
Ft. Belknap LDP																		
K526	NGS Dist. To K526			NGS Dist. To Cherry			NGS Dist. To Carson			Observed Dist. To K526			Observed Dist. To Cherry			Observed Dist. To Carson		
	Dist. METERS	Bearing		Dist. METERS	Bearing		Dist. METERS	Bearing		Dist. METERS	Bearing		Dist. METERS	Bearing		Dist. METERS	Bearing	
0.00	-	-	114705.75	N 24° 33' 35.2493" W	-	34516.25	S 43° 52' 17.9027" E	-	0.00	-	-	114755.54	N 22° 42' 22" W	-	114755.54	N 22° 42' 22" W	-	34540.55
38134.78	S 34° 24' 56.0783" W	-	99784.68	N 43° 40' 46.9845" W	-	57080.86	S 02° 23' 38.1260" E	-	38157.18	S 35° 08' 31" W	-	114705.75	N 24° 33' 35.2493" W	-	114705.75	N 24° 33' 35.2493" W	-	57117.42
114705.75	S 24° 33' 35.2493" E	-	0.00	-	-	147816.50	S 28° 16' 08.1271" E	-	114755.54	S 22° 42' 22" E	-	0.00	-	-	0.00	-	-	147888.51
Porter	114297.98	S 10° 47' 54.0631" W	-	67411.11	S 84° 00' 37.0572" W	-	137883.80	S 01° 16' 38.1850" E	-	114346.16	S 11° 31' 29" W	-	67424.81	S 84° 44' 30" E	-	137949.26	S 00° 00' 03" E	-
Lakeside	76382.23	S 41° 05' 56.5596" W	-	106796.63	N 64° 58' 09.3956" W	-	86530.54	S 17° 45' 50.3205" W	-	75422.68	S 41° 55' 19" W	-	106830.66	N 62° 18' 09" W	-	86581.58	S 18° 29' 04" W	-
Carson	34516.25	N 43° 52' 17.9027" W	-	147816.50	N 28° 16' 08.1271" W	-	0.00	-	-	34540.55	N 41° 24' 07" W	-	147888.51	N 26° 59' 58" W	-	0.00	-	-
Ft. Peck LDP																		
P354	NGS Dist. To P354			NGS Dist. To Richland			NGS Dist. To R540			Observed Dist. To P354			Observed Dist. To Richland			Observed Dist. To R540		
	Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing	
0.00	-	-	334578.61	N 10° 17' 19.9379" E	-	233797.03	N 57° 50' 30.1847" W	-	0.00	-	-	334703.87	N 13° 13' 31" E	-	233909.26	N 53° 12' 48" W	-	34540.55
MCCabe	485862.20	S 76° 46' 41.8597" W	-	467235.83	N 63° 49' 19.7255" W	-	667442.72	N 89° 38' 15.8402" W	-	486108.68	S 79° 42' 07" W	-	467387.88	N 59° 25' 28" W	-	667297.79	N 86° 25' 57" W	-
Richland	334578.61	S 10° 17' 19.9379" W	-	0.00	-	-	322627.52	S 51° 55' 01.6763" W	-	334703.87	S 13° 13' 31" W	-	0.00	-	-	322723.55	S 54° 51' 35" W	-
Madoc	376789.91	S 37° 54' 57.3588" W	-	174724.57	N 80° 32' 03.6019" W	-	457328.51	S 68° 34' 30.7046" W	-	376939.31	S 40° 50' 49" W	-	174761.88	N 76° 32' 13" W	-	457474.61	S 71° 30' 44" W	-
R540	233797.03	S 57° 50' 30.1847" E	-	322627.52	N 51° 55' 01.6763" E	-	0.00	-	-	233909.26	S 53° 12' 48" E	-	322723.55	N 54° 51' 35" E	-	0.00	-	-
Wind River LDP																		
Fort Washakie	NGS Dist. To Fort Washakie			NGS Dist. To J21			NGS Dist. To Fort Washakie			Observed Dist. To Fort Washakie			Observed Dist. To J21					
	Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing		Distance	Bearing				
0.00	-	-	110012.37	N 74° 41' 23.8554" E	-	89888.53	S 68° 12' 42.7071" E	-	0.00	-	-	0.00	-	-	0.00	-	-	-
P21	123374.22	N 00° 55' 45.2359" W	-	35518.71	N 30° 56' 40.6248" W	-	51321.81	S 51° 27' 18.7404" W	-	0.00	-	-	0.00	-	-	0.00	-	-
Pebble	110012.37	S 74° 41' 23.8554" W	-	0.00	-	-	67056.45	S 19° 57' 34.0004" W	-	0.00	-	-	0.00	-	-	0.00	-	-
Hart	74649.24	N 36° 00' 51.6898" W	-	108977.35	N 34° 49' 56.9270" E	-	47402.71	N 56° 07' 23.3946" E	-	0.00	-	-	0.00	-	-	0.00	-	-
J21	89888.53	N 68° 12' 42.7071" W	-	67056.45	N 19° 57' 34.0004" E	-	0.00	-	-	0.00	-	-	0.00	-	-	0.00	-	-

\*NOTE ALL DISTANCES ARE IN FEET UNLESS OTHERWISE NOTED